

STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	STATE DIST. NO.	COUNTY	SHEET NO.	TOTAL SHEETS

DESIGN DATA FOR 33" DEPTH ADJACENT BOX BEAM

SPAN LENGTH ϕ TO ϕ BEARING		50'-0"	52'-0"	54'-0"	56'-0"	58'-0"	60'-0"	62'-0"	64'-0"	66'-0"	68'-0"	70'-0"						
OVERALL LENGTH OF BEAM		51'-6"	53'-6"	55'-6"	57'-6"	59'-6"	61'-6"	63'-6"	65'-6"	67'-6"	69'-6"	71'-6"						
NO. OF 270 KSI, 1/2" ϕ LOW-RELAXATION STRANDS, AREA/STRAND = 0.167 SQ. IN.		12	12	12	14	14	14	16	16	18	18	18						
STRAND POSITION NUMBER	ROW 1	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14						
	ROW 2	15,16,27,28	15,16,27,28	15,16,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,19,20,23,24,27,28	15,16,19,20,23,24,27,28	15,16,19,20,23,24,27,28						
	ROW 3	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----						
	ROW 4	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34					
	PRESTRESSING FORCE IMMEDIATELY AFTER STRAND RELEASE, P_{pt} , (KIPS/BEAM)		394	394	394	457	458	458	521	521	583	584	585					
EFFECTIVE PRESTRESSING FORCE AFTER ALL LOSSES, P_{pe} , (KIPS/BEAM)		359	360	361	414	416	417	468	470	521	522	524						
REQUIRED FACTORED MOMENT @ STRENGTH I, M_u (FT-KIPS/BEAM)		858	918	979	1042	1107	1173	1244	1312	1383	1454	1527						
FACTORED FLEXURAL RESISTANCE, M_r (FT-KIPS/BEAM)		1092	1092	1092	1280	1280	1280	1478	1478	1656	1656	1656						
TOTAL NO. DEBONDED STRANDS		-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----						
DEBONDED STRAND POSITION NUMBER & SHIELDING LENGTH FROM EACH END	ROW 1	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----						
	ROW 2	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----						
NUMBER & LENGTH #4 ET TOP TENSION BARS @ EACH END		3 - #4 x 6'-6"	3 - #4 x 6'-6"	3 - #4 x 7'-0"	3 - #4 x 7'-0"	3 - #4 x 7'-0"	3 - #4 x 7'-6"	3 - #4 x 7'-6"	3 - #4 x 8'-0"	3 - #4 x 8'-0"	3 - #4 x 8'-0"	3 - #4 x 8'-6"						
NUMBER & LENGTH #5 BT BOTTOM TENSION BARS @ EACH END		6 - #6 x 7'-0"	6 - #6 x 6'-0"	6 - #6 x 7'-0"	6 - #6 x 8'-0"	6 - #6 x 8'-0"	6 - #6 x 8'-0"	6 - #5 x 8'-0"	6 - #6 x 8'-0"	4 - #6 x 9'-0"	4 - #6 x 9'-0"	6 - #6 x 9'-0"						
DESIGN CAMBER + = POSITIVE (UP) (INCHES)	@ RELEASE	0.19	0.18	0.17	0.27	0.26	0.25	0.39	0.38	0.51	0.50	0.48						
	@ ERECTION	0.25	0.22	0.19	0.35	0.32	0.27	0.49	0.44	0.66	0.59	0.52						
	@ FINAL	0.17	0.10	0.03	0.21	0.12	0.01	0.27	0.14	0.38	0.23	0.06						
NUMBER & SPACING OF TL-2 GUARDRAIL INSERTS SEE NOTE 6	NO OF INSERTS REQD.																	
	END OF BEAM TO ϕ OF FIRST INSERT EA. END																	
	ϕ OF 1st INSERT TO ϕ 2nd INSERT EA. END																	
WEIGHT OF TYPICAL BEAM INCLUDING DIAPHRAGM (TONS)		19.9	20.6	21.3	22.0	22.7	23.4	24.4	25.1	25.8	26.5	27.2						

MIN. CONCRETE STRENGTH @ RELEASE	= 5500 PSI
MIN. CONCRETE STRENGTH @ 28 DAYS	= 8000 PSI
INITIAL PULL/STRAND	= 33,820 LBS
CROSS-SECTION AREA/STRAND	= 0.167 SQ.IN.

NOTES

- BEAM WEIGHTS LISTED IN THE DESIGN TABLE ARE BASED ON ZERO SKEW, 2 FT. LONG ENDBLOCK AND DIAPHRAGMS SPACED @ 15 FT C/C. WEIGHTS FOR SKEWED BEAMS, LONGER ENDBLOCKS AND ADDITIONAL DIAPHRAGMS SHOULD BE ADJUSTED ACCORDINGLY.
FOR ADDITIONAL DIAPHRAGMS, ADD 497 LBS/DIAPHRAGM.
FOR SKEW ADD 33 LBS/DEGREE OF SKEW/END.
FOR LONGER ENDBLOCK, ADD 596 LBS/LF/END.
- DESIGNERS SHOULD NOTE THAT DATA IN STANDARD TABLE IS BASED ON EVEN SPAN LENGTHS, A TWO LANE STRUCTURE 8 BEAMS WIDE AND ZERO SKEW. SUPERIMPOSED DEAD LOADS INCLUDE TYPE F PARAPET (321 PLF) AND A FWS OF 50 PSF. FOR NON-STANDARD BRIDGES DATA SHOULD BE VERIFIED AND IF REQUIRED NEW DESIGN DATA ENTERED INTO BLANK COLUMNS. IN NO CASE SHALL THE STANDARD DESIGN TABLE BE ALTERED.
- IF BEAM DOES NOT MEET ALL TOLERANCES REFER TO MP 603.10.40 FOR GUIDANCE. MEASUREMENT OF CAMBER FOR COMPARISON TO PREDICTED DESIGN VALUES SHOULD BE COMPLETED WITHIN 72 HOURS OF RELEASE. ADDITIONALLY, CAMBER SHOULD BE EVALUATED UNDER CONDITIONS THAT MINIMIZE THE EFFECT OF TEMPERATURE VARIATION.

- DESIGNER, FABRICATOR, AND ERECTOR SHALL BE AWARE THAT SKEWED END BEAMS MAY TWIST OR WARP, CAUSING UNEVEN BEAM SEATING AT THE BEARINGS. THE CONTRACTOR IS REQUIRED TO CORRECT AT THE TIME OF ERECTION, BEFORE THE BEAMS ARE SECURED IN PLACE. METHOD OF CORRECTION SHALL PROVIDE AN EVEN, TOTAL BEARING AND A LEVEL TOP BEAM SURFACE. TOLERANCE, AFTER CORRECTION, SHALL BE (+/-) 1/8 INCH. THE FABRICATOR SHALL NOTIFY THE CONTRACTOR AND DESIGNER IF CORRECTIONS ARE REQUIRED PRIOR TO SHIPMENT.
- MAXIMUM BEAM SKEW SHALL BE 30 DEGREES.
- DESIGNER INPUT VALUES OF NUMBER OF INSERTS, DISTANCE FROM END OF BEAM TO ϕ FIRST INSERT, AND ϕ FIRST INSERT TO ϕ SECOND INSERT. ABOVE VALUES SHALL BE BASED ON THE REQUIRED 6'-3" GUARDRAIL POST SPACING ACROSS THE BRIDGE.
- THIS SHEET SHALL BE USED IN CONJUNCTION WITH STANDARD SHEETS BR-B33A, BR-B100, BR-B101, BR-B102A & B, BR-B103, BR-B104, BR-B105A & B AND BR-B106 AS APPLICABLE.

APPROVED: _____ DIRECTOR, ENGINEERING DIVISION	DATE: _____
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS ENGINEERING DIVISION	
DESIGN TABLE FOR 33" PRESTRESSED BOX BEAM	PREPARED: 07-02-07
REVISED: 7-10 TW	
REVISED STANDARD SHEET BR-B33B	

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS ENGINEERING DIVISION	
DESIGNED BY:TW/	
DRAWN BY:TW/	
CHECKED BY:TW/	
REVIEWED BY:TW/	
DATE:	
SCALE:	
SHEET NO OF	
BROGE NUMBER	
DESIGN TABLE FOR 33" PRESTRESSED BOX BEAM	