

Hot-Mix Asphalt Construction

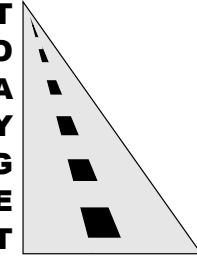


Federal Highway
Administration



**JOINT
AASHTO
FHWA
INDUSTRY
TRAINING
COMMITTEE
ON ASPHALT**

"Partners in Performance"



1-1

Presentation Format

- Lecture/discussion
- Protocol:
 - Informal
 - Questions are encouraged
 - Class participation is essential
 - Respect others!
 - Observe class schedule



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Module 1

Course Introduction, Communication, and Project Documents

1-3

Need for the Course

(Why are you here?)

- Training for individuals assigned to asphalt paving projects
- Asphalt construction isn't simple!
- Confidence in work
- More cost effective use of tax dollars spent on asphalt pavements
- FHWA requirement!
- One person on paving crew certified

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From the 2017 Standard Specifications

- The Contractor shall maintain necessary equipment and qualified personnel including at least one certified Asphalt Field and Compaction Technician at each project during paving operations.
- Additionally, a certified Asphalt Field and Compaction Technician with certification to perform nuclear density testing of asphalt pavements shall perform all testing necessary to assure compaction of the asphalt meets specification requirements.

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Overall Course Objectives

1. Describe the purpose of project documents and cooperative communication on the job
2. List the steps involved in preparing bases and existing surfaces for asphalt overlays
3. Define a proper asphalt delivery process to the job site
4. Explain the effect of the various components of an asphalt paving machine on the finished mat
5. Describe what effect the compaction process has on the finished pavement

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Overall Course Objectives

6. Identify the roles and responsibilities of the WV DOH Inspector
7. Describe how density measurements are taken
8. Explain the different processes of QC/QA for testing
9. Define the key components of PWL paving
10. Describe the process for troubleshooting if things go wrong

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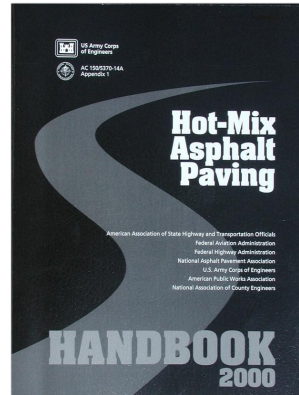
Local Issues

- Typical design
 - Superpave? Marshall? SMA? OGFC?
- Contractors/material suppliers
 - Aggregate source? Skid??
- Specs...401, 410?
- Hauling
- Laydown
- Compaction
- Other?

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Course Reference Materials

- Participant's Workbook
- Hot-Mix Asphalt Paving Handbook 2000
- WVDOT Materials
 - MP 401.05.20
 - Compaction Worksheets 401 & 407
 - Daily Work Report
 - Tack Coat Form (SM)



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Safety



1-10

Communication

- Schedule
- Weather forecast
- Closed Lanes
- Traffic Control
- Plant issues
- Other requirements/restrictions
- CHANGES



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Project Documents

- | | |
|------------------------|---------------------------|
| • Reports | • Change Orders |
| • Pay Estimates | • As-Built Plans |
| • Traffic Control Plan | • Job Mix Formula |
| • Compaction Forms | • Environmental Documents |
| • Tack Coat Forms | • QC Plan |
| • Daily Work Report | |

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Project Documents

Hierarchy

1. Special Provisions
2. Plans
3. Supplemental Specifications
4. Standard Specifications

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Project Documents

1. Special Provisions
 - Project specific additions or revisions to the standard or supplemental specifications
2. Plans
 - Drawings of location, character, dimensions, and details of work
 - Plan notes

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Project Documents

3. Supplemental Specifications
 - Approved additions and/or revisions to standard specifications
 - Typos are issued as errata

1-15

Project Documents

4. Standard Specifications
 - Directions, provisions, and requirements for performing the work illustrated and described in the plans
 - Methods of performing the work, desired outcome, or qualities and quantities of materials and labor to be furnished

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West Virginia Challenges

- WV is one of only four states that take care of both state and county routes
 - Delaware, Virginia, and North Carolina are the others
 - WV has the 6th largest state-maintained highway system in the nation
 - DOH owns 36,000 miles...24,500 miles are paved
 - LOWEST total \$ Disbursements/mile
 - US Avg: \$178,000
 - WV: \$35,000
 - Highest Percentage of narrow lanes
 - US Avg: 10%
 - WV: 52%
- Source: [23rd Annual Highway Report on the Performance of State Highway Systems, 2018](#)

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The “Team”

- Owners
 - Long-term performance
 - Ensure quality
- HMA Industry
 - Provide quality
 - Increase performance
 - Lower costs

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Teamwork and Cooperation



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Questions

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Quality Control/ Quality Assurance

Learning Objectives

1. Describe and differentiate between quality control (QC) and quality assurance (QA)
2. Relate different types of specifications to pavement performance and to risk
3. Identify sources of variability within HMA construction process
4. Explain the use of statistical analysis of test results for QC and QA

What does QC/QA mean?

Quality Control

- QC refers to the **control** component of the production or construction process
- Also referred to as **process control**
- QC **ensures** the production of uniform materials that meet specification
- QC is achieved through **periodic inspection and testing**
- QC is the responsibility of the producer or contractor!

Quality Assurance

- QA refers to the **assurance** portion of the overall inspection process
- QA **assures** the buyer/owner that the producer's test results are accurate
- Sampling and testing for acceptance are typically conducted at less **frequency** and with greater **randomization** than the producer's process
- QA is the responsibility of the buyer!

Elements of QA



Elements of QA



Producer / Contractor provides documentation that the material / end product met specification.

Elements of QA



Buyer / Owner determines if the quality of the product as specified in the contract requirements

Elements of QA



Activities that combine to produce an unbiased and independent evaluation of all the sampling and testing procedures used in the acceptance program

QA in Various Settings

Buyer/Owner	Product	QC Process	Acceptance	Independent Assurance
Individual	New Car	?	?	?
Individual	Fast Food	?	?	?
Public Agency	Road-way	?	?	?

Specifications

- Types
- Relationship to performance
- Risk considerations

Types of Specifications

- Method (Recipe)
- QC/QA
- End-Result
- Performance

Method Specifications

- Old school
- Owner prepares designs and sets requirements for structure, materials and construction processes
- Contractor supplies manpower, materials and equipment
- Owner is responsible for inspection
- Payment based on labor, materials, and equipment use

Method Specifications

- Problems:
 - Little or no testing
 - Question of quality
 - Uncertainty of performance

End-Result Specifications

- Owner specifies the expected end-result of the finished pavement such as:
 - Layer thickness
 - Mix properties
 - Smoothness



- Establishes criteria for acceptance (including rejection and pay adjustment)

End-Result Specifications

- Owner does NOT specify equipment nor methods of construction
- Increased testing, statistically rigorous, and considers owner and contractor risk
- Requires both QC and QA
- Third party (for independent assurance) often required
- Commonly used

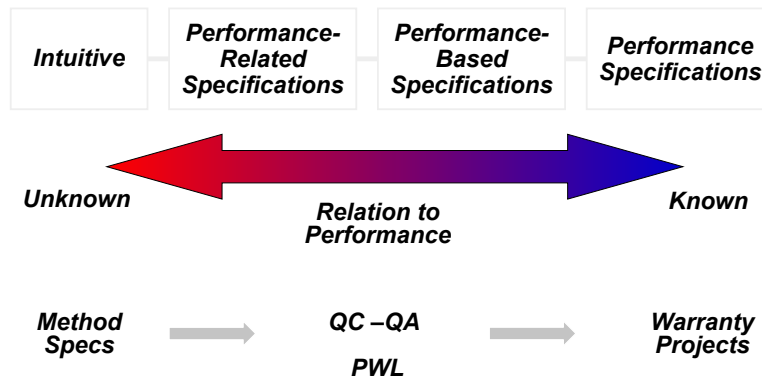
QC/QA Specifications

- Lies between method and end-result specifications
- Separates QC from QA
- Applies statistical approach to both QC and QA
- Requires inspection and testing
- Introduced pay adjustment as a function of QA testing
- Gaining popularity

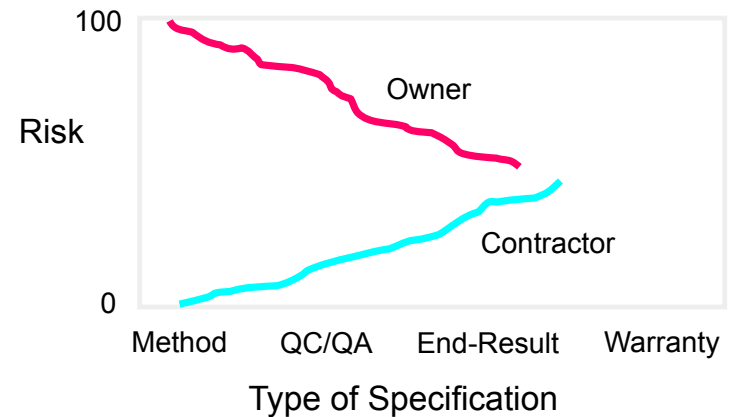
Performance Specifications

- Performance-related
- Performance-based
- Guarantee and Warranty

Relationship to Performance

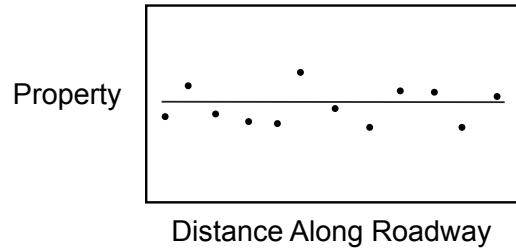


Risk Considerations



Material and Construction Variability

- What is it?



- Importance
- Properties affected
- Standard measures
- Sources

Importance of Variability

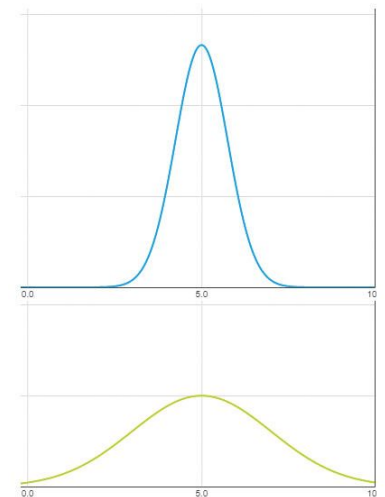
- Major effect on:
 - Quality control
 - Quality assurance
 - Pay adjustment
 - Pavement performance

Key Properties Affected by Construction Variability

- Thickness
- Aggregate gradation
- Asphalt content
- Air void content (or density)
- Smoothness

Standard Measures of Variability

- Range
- Standard deviation



Sources of Variability

- Materials and construction:
 - Plant (stockpiles, cold feeds, binder addition, additives, mixing, transfer and storage)
 - Hauling (loading, transport, unloading)
 - Paver (operation)
 - Compaction (roller equipment and patterns)
- Sampling and testing....

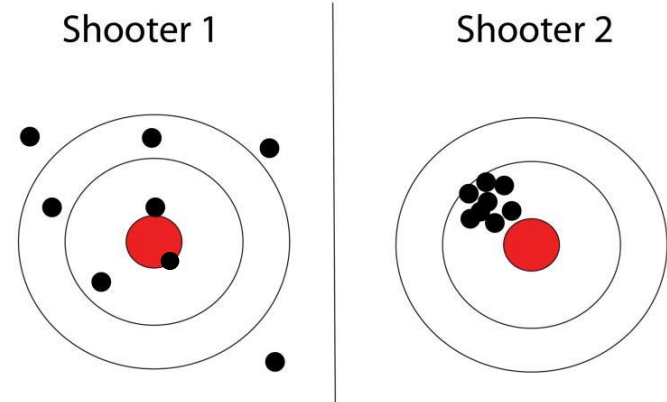
Reduce Sampling and Testing Variability

- Technician Training
- Certification (Qualified Workforce)
- Laboratory Accreditation (AASHTO Re:Source)
- Regionalize/Standardize Test Methods
- Regionalize/Standardize Test Method Options
- Proficiency Sample Programs (Round Robins)

Use of Statistical Analysis for Quality Control

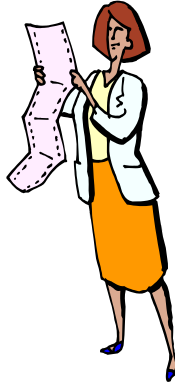


Who is the better shooter?



Control Charts

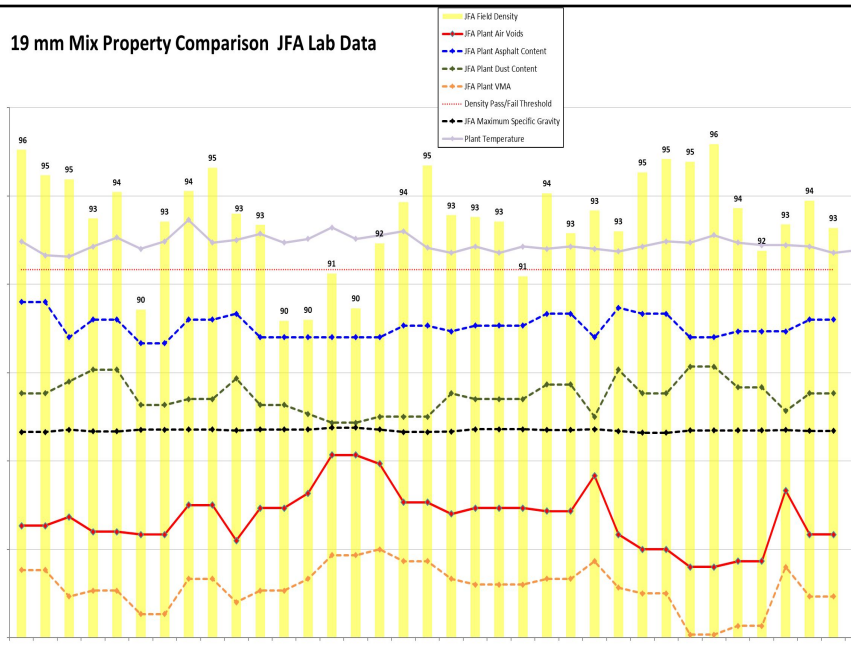
- What is plotted?
 - Control sieves
 - Asphalt content
 - Specific gravities
 - Voids and VMA
 - In-place density
 - Layer thickness



Benefits of Control Charts

- Early detection of trends
- Establish process capability
- Decrease inspection frequency
- Permanent record of quality
- Provide a basis for acceptance
- Instill quality awareness
- Taking corrective measures
- Evaluating data for cost savings
- Recording and reporting

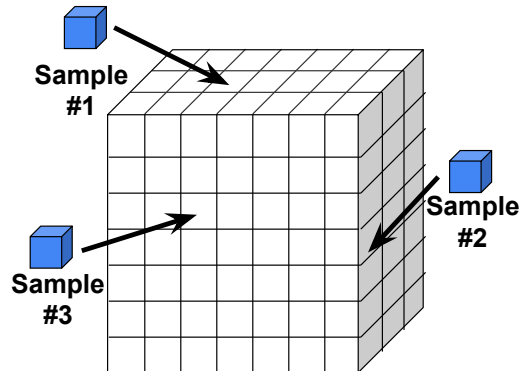
19 mm Mix Property Comparison JFA Lab Data



Use of Statistical Analysis for Quality Assurance

- Sampling
- Testing
- Apply acceptance criteria
- Determine pay adjustment

Sampling

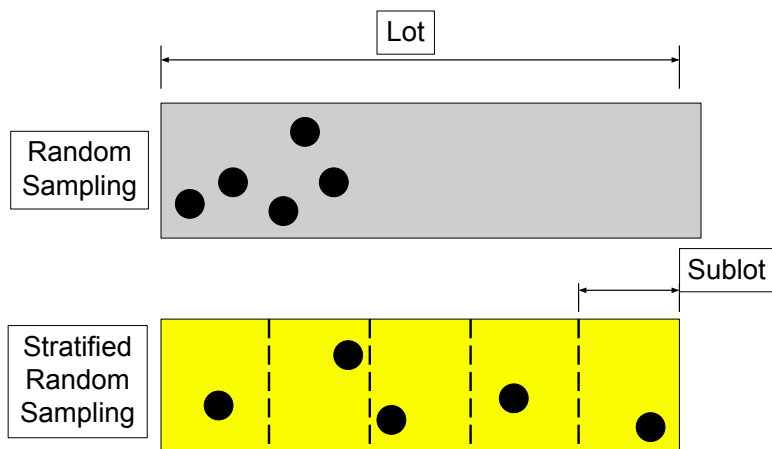


Random Sampling

- Any portion of the population has equal chance of being selected
- Bias is introduced when judgment is used
- Use random number tables



Lots and Sublots



Pay Adjustment

- Most QA processes apply pay factor to adjust payment on a lot-by-lot basis
 - PWL and/or PD are commonly used to calculate Pay Factors (both penalty and bonus)
 - Pay Factors are typically determined for different factors and then combined
 - Max and limits on Pay Factors are established
- Not common to consider the Contractor's QC process

Payment Based on PWL

- Small number of tests results outside the specification limits is normal and not necessarily detrimental to performance
- Can also define Acceptable Quality Level (AQL) for key M&C factors (within which no pay adjustment is made)

Summary

1. Differentiate between quality control (QC) and quality assurance (QA)
2. Different types of specifications (relate to performance and risk)
3. Sources of variability within HMA construction process (emphasis on M&C variability)
4. Statistical analysis for QC (control charts) and QA (sampling to payment)

Questions?

Module 2

Preparation for Paving

Module Outline

- Preparation for Paving
 - New Construction
 - Overlays
 - Asphalt
 - PC Concrete
- Tack Coats
 - Introduction
 - Materials
 - Application
 - Verification

Preparation for Paving

- Often doesn't get due consideration
- It is often time consuming and labor intensive
- Asphalt layers cover up the potential problems
- THE PROBLEMS WE DO NOT FIX TODAY WILL NOT GO AWAY
 - Often the problems get worse
 - And more costly to fix the second time

Preparation for Paving

- The most common surfaces overlaid:
 - Subgrade
 - Granular Base Course (Aggregate Base)
 - Existing Asphalt Pavement
 - Existing Portland Cement Concrete Pavement
- Each have their own unique requirements

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New Construction



- Subgrade Preparation
 - The foundation
 - Thickness design uses soil type
 - Support all three stages
 - Construction equipment
 - Pavement
 - Anticipated traffic
 - Must be properly graded
 - Transversely & longitudinally
 - For drainage
 - Cross slope & smoothness
 - Uniformly compacted density



New Construction



- Aggregate Base Preparation
 - Mix to proper moisture content
 - Best Practice:
 - Use an old paver
 - 4 to 10-inch compacted lifts
 - Stagger joints
 - Longitudinal and transverse
 - At least one-foot in each layer
 - Compact to percentage of Proctor specified



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Preparing Existing Asphalt



May be as simple as:

- Sweeping
 - Multiple passes may be necessary
- Applying tack coat



Surface Preparation



Thoroughly clean the surface

Preparing to Overlay Asphalt



Patching

- DOH responsible for patching
- Contractor responsible for overlay
 - Patch & level corrections on isolated local spots

Cleaning and filling cracks

- Remove cold patch material

Placing a leveling course

- 1" thick Scratch Course

Milling the surface

Failed areas MUST be cleaned, repaired to good structural condition before overlaying

Asphalt Patching Material



Hot Mix Asphalt (HMA)

- Long lasting patches when correctly constructed
- Use WV DOT:
 - Wear 1 NMAS 3/8
 - Wear 4 NMAS 3/4
- Use appropriate PG-binder



Standardized Aggregate Sizes



Nominal Maximum Aggregate Size (NMAS) - One sieve size larger than the first sieve to retain more than 10 percent

Maximum Aggregate Size (MAS) - one sieve size larger than the NMAS

<u>Sieve Size</u>	<u>Percent Passing</u>	
3/4"	100	- MAS
1/2"	94	- NMAS
3/8"	89	<i>1st sieve to retain cumulative 10% or greater</i>
No. 4	60	
No. 8	35	
No. 16	23	
No. 30	13	
No. 50	9	
No. 100	6	
No. 200	4.8	

Full Depth and Deep Patching



Definitions:

- Full depth patching
 - Removal & replacement entire pavement cross section
 - Down to the subgrade or granular base
- Deep Patching
 - Removal & replacement of 4 inches of asphalt
 - Not all of the way to the subgrade or granular base

Full Depth and Deep Patching



Applications:

- Full depth patching
 - Applicable
 - Flexible (asphalt) pavements
 - Rigid (PC Concrete) pavements
- Deep patching
 - When patching with asphalt
 - Applicable to flexible pavements only

Both types of patches are intended to be permanent

Full Depth and Deep Patching



Step 1:

- Remove distressed material to firm support:
 - May include removing some of the subgrade
 - Excavation should extend at least 1-foot into existing pavement



Full Depth and Deep Patching



Step 1 (continued):

Patches:

- Square-edged and rectangular in shape
- Clean, vertical faces
 - Using a pavement saw
 - Chisel faces to vertical below sawcut
- If approaches width of lane
 - Patch the full lane

Full Depth and Deep Patching



Irregular patch

- Compaction is difficult



Straight rectangular lines

- No distress outside patch

What's Wrong Here?



Full Depth and Deep Patching



Expedite removal



Medium mill for large patches

Skid steer mill for small patches



Full Depth and Deep Patching



Step 2:

- After cleaning
 - Recompact Agg base or subgrade material
 - Tack vertical faces
 - Deep patching
 - Tack bottom surface



Full Depth and Deep Patching



Step 3:

- Using a dense-graded mix
 - Placed in small piles to avoid excessive handling
 - Patches >6 in deep place lifts less than 4 inches
 - Compact each lift thoroughly



Full Depth and Deep Patching



Step 4:

- Compact the patch thoroughly
 - Flush with surrounding pavt
- Vibrating plate compactor for small patches & corners
- Match rollers to patches large enough to accommodate it
- Make sure corners are compacted



Full Depth Patching



Full Depth Patching



Full Depth Patching



Full Depth Patching



Full Depth Patching



Surface lift

Full Depth Patching



Full Depth Patching



Full Depth Patching



Full Depth Patching



Full Depth and Deep Patching



Step 5:

- Check the smoothness with a straightedge
- Do not overfill in anticipation of traffic compaction



Cold Milling



Advantages of Cold Milling



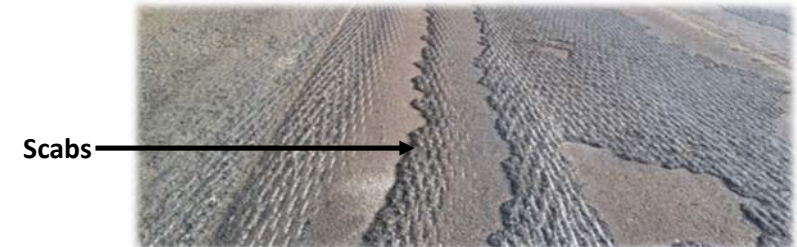
- Efficiently removes deteriorated pavement
- Provides opportunity to improve smoothness
- Provides RAP for recycling operations
- Increased surface texture improves bonding



Cold Milling



- Mill below distress (rutting, raveling, top-down cracking)
- Don't leave "scabs" of asphalt
 - Avoid milling to within ½ inch of interface
- Consider properties of existing asphalt before milling
 - Increasing value of RAP obtained



Cold Milling



- How will grade be controlled?
 - Ski, string line, laser, other
- Will ride quality be measured on the milled surface?
 - Profilograph, profiler, none

Cold Milling



Surface texture is a function of:

- Carbide bit spacing and condition
- Depth of cut
- Rotational speed of cutting head
- Speed of travel
- Condition of moldboard
 - Milling strike-off



Cleaning the Surface



- Goals:
 - Free of debris
 - Minimal or no dust
 - Dry surface
- Ready for tack coat



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Preparing to Overlay Existing PCC



- Full depth replacement of distressed slabs
 - Asphalt or PCC patch
 - Correct problems in base/subgrade
- Spalled joints repaired partial depth
 - Use PCC for patching
- Stabilize rocking slabs
- Replace joint sealer as required
- Clean and tack surface

Above precautions often don't work long-term

Preparing to Overlay Existing PCC



A better way to handle PCC pavement:

- Cracking and Seating
- Breaking and Seating
- Rubblization
- Ideal for PCC that is not structurally sound
- Cost effective alternative to total reconstruction
 - Utilizes all material layers in-place
 - Reduces the size subject to movement
 - Makes them easier to seat and stabilize

Cracking/Breaking and Seating



- Reduces slab length (2-5 feet) by inducing fine vertical cracks
- Seats broken slabs against subgrade by rolling
- Crack and Seat applies to jointed plain concrete
 - Very good performance history
- Break and Seat applies to jointed reinforced concrete
 - Must rupture bond between reinforcing steel and PCC
- Rubblization is preferred due to variable performance history of break/seat (bond not always broken)

Cracking/Breaking and Seating Process



- Remove existing overlay
- Correct drainage problems
- Crack PCC slabs
 - “Guillotine” hammer
- Seat cracked PCC
 - 35-50 ton pneumatic roller
- Remove/patch any soft areas identified
- Apply structural overlay

Benefits of Crack/Break and Seat



- No hauling or disposal costs since none of PCC is discarded
- Existing PCC stays in place to serve as base for the new overlay
- Saves natural resources, landfill space, environmentally friendly
- Expedites construction time
- Weather delays minimized since subgrade never exposed
- Cost effective rehabilitation technique

What is Rubblization?



- “Rubblizes” PCC slabs into high quality aggregate base
- Completely eliminates slab action & other distresses
 - Reflective cracking
 - D-cracking and ASR
 - Slab rocking, pumping, curling, etc.
- Destroys bond between concrete and steel
- Converts failed rigid system into new flexible one
- Two distinct methods and equipment types:
 - Multiple Head Breaker (MHB)
 - Resonant Pavement Breaker (RPB)

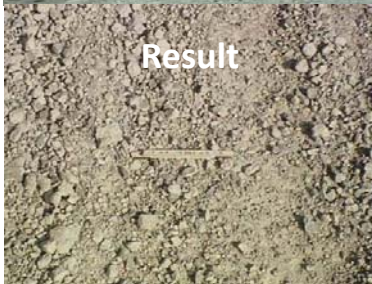
Resonant Pavement
Breaker (RPB)



Roll shattered PCC



Result



Place overlay



Multi Head Breaker
(MHB)



Roll MHB broken PCC



Z-Grid Roller
Processing
Rubbleized Surface



Place HMA



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Definition



Tack Coat—sprayed application of asphalt cement upon an existing asphalt or Portland cement concrete pavement which may or may not have been milled before an overlay, or between layers of fresh asphalt concrete.

Tack Coat



Why do we use Tack Coat?

- Promotes bond between layers
- Prevents slippage
- Provides additional moisture barrier long transverse & longitudinal joints

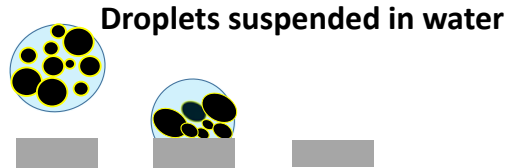


Tack Coat Definitions



- **Undiluted Emulsion** - primarily an asphalt binder, water, and an emulsifying agent
- **Diluted Emulsion** - emulsion with additional water
 - Most common dilution rate is 1:1
 - One-part undiluted emulsion and one-part additional water
- **Residual Asphalt** - remaining asphalt after an emulsion has set
 - Typically 57-70 percent undiluted emulsion
- **Tack Coat Break** - when water separates from the asphalt to change color from brown to black
- **Tack Coat Set** - when water has evaporated, leaving only the residual asphalt. Some refer as completely broken

Emulsion Breaking & Curing



- Breaking
 - Contact with surface changes pH; reducing charge
- Setting & Curing
 - Evaporation leads to coalescence
 - Original asphalt characteristics return

What's wrong (if anything) with the following specification regarding application rate?:

"Apply the tack coat at a rate of 0.05 gallons/yd²"

What difference does it make?



If the example spec *intended* 0.05 **gal/yd²** of residual asphalt:

*Undiluted emulsion applied at 0.05 gal/yd²
using an emulsion with 60% residual asphalt,
leaves **0.03 gal/yd²** on the roadway.*

40% less than intended

What difference does it make?



If the example spec *intended* 0.05 **gal/yd²** of residual asphalt:

*Diluted Emulsion using the same emulsion diluted 1:1
applied at 0.05 gal/yd² leaves
0.015 gal/yd² on the roadway.*

70% less than intended

What difference does it make?



If the example spec *intended* 0.05 **gal/yd²** of residual asphalt:

*To get Residual Asphalt at 0.05 gal/yd²
using an emulsion with 60% residual asphalt
would need to apply a “shot rate” of:*

**0.083 gal/yd² of Original Emulsion or
0.167 gal/yd² of 1:1 Diluted Emulsion**

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Top 5 Emulsions Used



1.CSS-1h	(52%)	Percentage of material used, as self- reported by states in NCHRP Synthesis 516 Survey
2.Non-standard*	(48%)	
3.SS-1h	(46%)	
4.SS-1	(36%)	
5.CSS-1	(28%)	

* *Non-standard means that a state has come up with its own nomenclature for an emulsion, outside those specified in AASHTO M 140 (Anionic Emulsions), M 208 (Cationic Emulsions), or M 316 (Polymer-Modified Cationic Emulsions)*

Examples: SS-1hp, CRS-2h, EBL

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Typical Requirements



- Weather
 - Minimum +40°F
 - Extra care if < +50°F
 - Fog or rain not imminent
- Application
 - Swept surface
 - Prescribed application rate
 - 10% maximum variation
 - Drying time set by engineer



Tack Coat



Apply past full-width of mat to minimize movement of unsupported edge



Tack not wide enough



Application Rates?



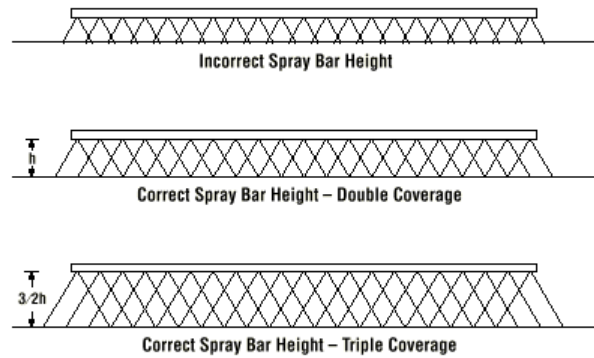
- What is the Optimal Application Rate?
 - Depends on surface Type & Condition
 - Application rate must be clearly stated in terms of:
 - Residual, Undiluted, or Diluted condition

Recommended Ranges

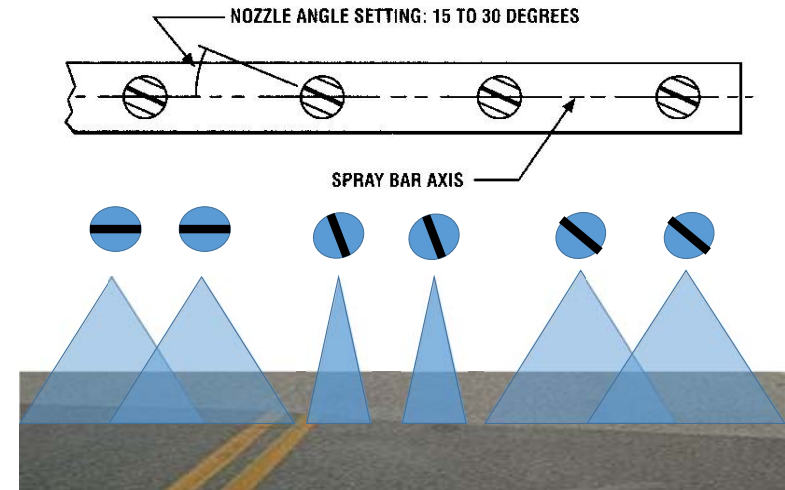
Surface Type	Residual Rate (gsy)	Appx. Bar Rate Undiluted* (gsy)	Appx. Bar Rate Diluted 1:1* (gsy)
New Asphalt	0.02 – 0.05	0.03 – 0.07	0.06 – 0.14
Existing Asphalt	0.04 – 0.07	0.06 – 0.11	0.12 – 0.22
Milled Surface	0.04 – 0.08	0.06 – 0.12	0.12 – 0.24
Portland Cement Concrete	0.03 – 0.05	0.05 – 0.08	0.10 – 0.16

*Assume emulsion is 33% water and 67% asphalt.

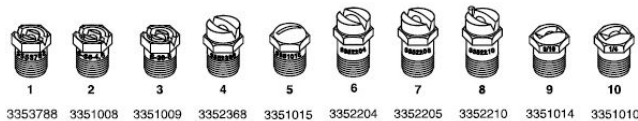
Spray Bar Height and Coverage



Spray Bar & Nozzles



Etnyre Spraybar Nozzles



Ref.	Part No.	Description	Application Gallons Per Square Yard	Application (Metric) Liters Per Square Meter	US Flow Gallons Per Minute Per Foot
1	3353788	V Slot Tack Nozzle	.05 - .20	.23 - .91	3.0 to 4.5
2	3351008	S36-4 V Slot	.10 - .35	.45 - 1.58	4.0 to 7.5
3	3351009	S36-5 V Slot	.18 - .45	.81 - 2.04	7.0 to 10.0
4	3352368	Multi-Material V Slot	.15 - .40	.68 - 1.81	6.0 to 9.0
5	3351015	3/32" Coin Slot	.15 - .40	.68 - 1.81	6.0 to 9.0
6	3352204*	Multi-Material V Slot	.35 - .95	1.58 - 4.30	12.0 to 21.0
7	3352205*	Multi-Material V Slot	.20 - .55	.91 - 2.49	7.5 to 12.0
8	3352210	End Nozzle (3352205)	.20 - .55	.91 - 2.49	7.5 to 12.0
9	3351014	3/16" Coin Slot	.35 - .95	1.58 - 4.30	12.0 to 21.0
10	3351010	1/4" Coin Slot	.40 - 1.10	1.81 - 4.98	15.0 to 24.0

* Recommended nozzles for seal and chip with emulsified asphalts.

68

Nozzle Selection



69





Nicely tacked

Module Outline

- Preparation for Paving
 - New Construction
 - Overlays
 - Asphalt
 - PC Concrete
- Tack Coats
 - Introduction
 - Materials
 - Application
 - Verification

Application Calculations

Calculating field application rates

- Three primary methods of determining field application rates
 - Determination by volume
 - Determination by weight or mass
 - Determination by direct measurement, ASTM D2995

Calculating rates by Volume



- Rate of material applied is calculated by determining the volume of material distributed
- Either by:
 - Observation & record of an onboard volume meter or gauge
 - Using a tank stick method where the depth of material is measured in the tank
- Volume is calculated with the use of a calibration table



Correcting For Temperature



- Asphalt & water expand & contracts when temperatures deviate from 60°F.
- As temperatures rise above 60°F
 - Expansion occurs and the resulting
 - Density (#/gal.) decreases
- As temperatures cool below 60°F contraction occurs and the density increases
- A Temperature–Volume correction table for asphalt emulsion is available in MS-19, page 91

Table 13. Temperature - volume corrections for asphalt emulsions ⁽⁶⁾.

°C	°F	M	°C	°F	M	°C	°F	M
10.0	50	1.0025	35.0	95	0.9912	60.0	140	0.9800
10.6	51	1.0022	35.6	96	0.9910	60.6	141	0.9797
11.1	52	1.0020	36.1	97	0.9907	61.1	142	0.9795
11.7	53	1.0017	36.7	98	0.9905	61.7	143	0.9792
12.2	54	1.0015	37.2	99	0.9902	62.2	144	0.9790
12.8	55	1.0012	37.8	100	0.9900	62.8	145	0.9787
13.3	56							0.9785
13.9	57							0.9782
14.4	58	1.0005	39.4	103	0.9892	64.4	148	0.9780
15.0	59							0.9777
15.6	60							0.9775
16.1	61	0.9997	41.1	106	0.9885	66.1	151	0.9772
16.7	62	0.9995	41.7	107	0.9882	66.7	152	0.9770
17.2	63	0.9992	42.2	108	0.9880	67.2	153	0.9767
17.8	64	0.9990	42.8	109	0.9877	67.8	154	0.9765
18.3	65	0.9987	43.3	110	0.9875	68.3	155	0.9762
18.9	66	0.9985	43.9	111	0.9872	68.9	156	0.9760
19.4	67	0.9982	44.4	112	0.9870	69.4	157	0.9757
20.0	68	0.9980	45.0	113	0.9867	70.0	158	0.9755
20.6	69	0.9977	45.6	114	0.9865	70.6	159	0.9752
21.1	70	0.9975	46.1	115	0.9862	71.1	160	0.9750

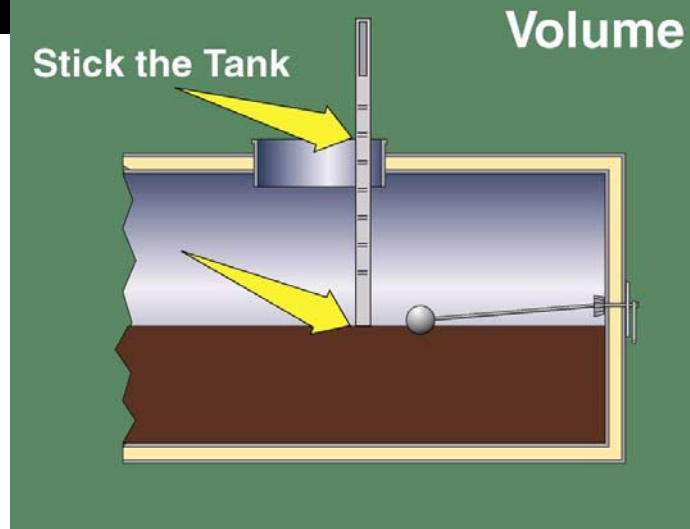
$$Volume_{@^{\circ}F} \times M_{value} = 60^{\circ}F Vol.$$

$$Volume_{@^{\circ}C} \times M_{value} = 15.6^{\circ}C Vol.$$

Dipstick Method



1. Measure Emulsion level & use calibration chart to convert to volume
2. Record Emulsion Temperature
3. Spray Tack Coat Over a Known Area
4. Measure Emulsion level & use calibration chart to convert to volume
5. Subtract volume Step 4 from volume Step 1
6. Correct Volume for Temperature Variation from 60°F



Distributor must be level when checking volume using a measuring stick



Dipstick Equation:

$$\frac{9 \times \text{Gallons Applied}}{\text{Width} \times \text{Length}}$$

(feet) (feet)

Note: 9 to convert from square feet to square yards
Use as required.

Comments on Calculating by Volume

Pros:

- Quick
- Simple
- Accuracy improves with larger areas

Cons:

- Volume requires
 - Dip Stick, or
 - Volumeter
- Dilution rate vital
- Temperature correction required
- Inaccurate on small areas

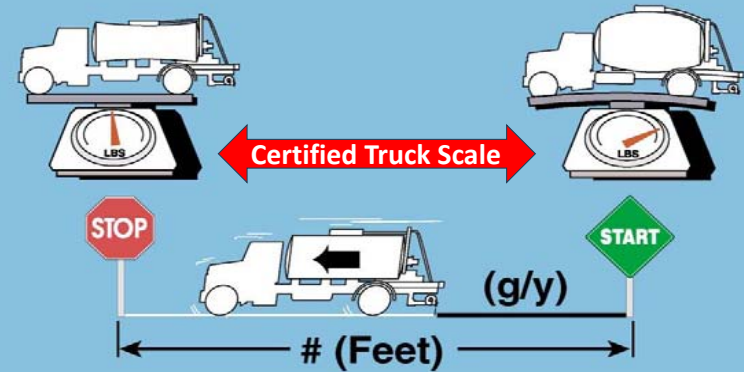
Calculating rates by Weight (Mass)



Calculating an application rate by weight:

- Most accurate method
- Supplier's Bill of Lading contains a 60°F weight per gallon
- Weight measurements are not affected by temperature
- Constant weighing after each shot can be complicated
- Recommend using this method:
 - Full load applications
 - Calibration

Weight



Key Items for Inspectors



- Check truck setup
 - Spray bar height (~12")
 - Appropriate nozzles
 - Uniform orientation
 - From 15-30°
 - Check application rate gauge in truck
 - Check application temperature
- Collect samples
- Know the desired application and residual rates
- Visually inspect application
- Verify application
 - Volume
 - Mass

Module 3 Delivery

Module Outline

- **Transportation and delivery**
 - Introduction
 - Truck types & requirements
 - Mix delivery concerns

The Hauling Operation

Hauling operation must:

- Provide a steady and consistent flow of mixture
- From the plant to the paver

This is accomplished by:

- *Timely and proper loading of trucks at plant*
- *Adequate number of trucks to support continuous paver speed*
- *Consistent use of proper techniques for loading and unloading*
- *Delivered mix meets temperature and non-segregation requirements*

Contractor must anticipate the activities to properly estimate cycle time

Typical Truck Requirements

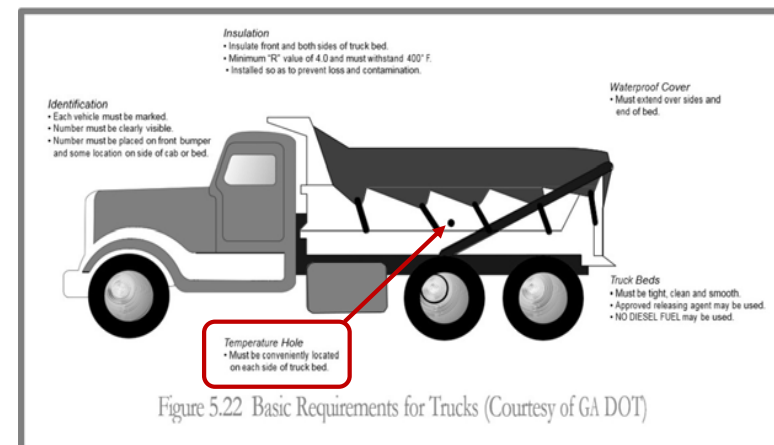


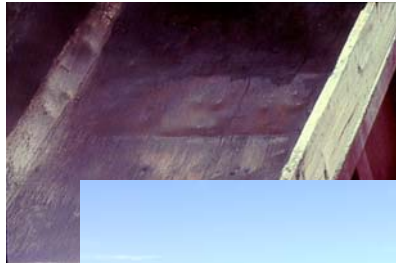
Figure 5.22 Basic Requirements for Trucks (Courtesy of GA DOT)



Truck Bed Requirements—Release Agents



- Tight, clean, smooth metal beds
- Thinly coated with min amount:
 - Soap solution
 - Lime solution
 - Other approved release agent
- Do not allow ponding of solutions in the truck bed



No diesel fuel or other contaminating solvents allowed!



Module Outline



- Transportation and delivery
 - Introduction
 - Truck types & requirements
 - Mix delivery concerns

Types of Trucks



End dump:

Typical capacity 13-15 tons

- Lowest capacity
- Shorter wheel base makes it more maneuverable
- Good for jobs in tight spaces
- Watch for overhead obstructions



Types of Trucks



Semi-trailer, high dump:

Typical capacity 20-22 tons

- Larger capacity
- Easier to segregate with improper loading
- Watch for overhead obstructions



Module Outline



- Transportation and delivery
 - Introduction
 - Truck types & requirements
 - Mix delivery concerns

Tarping Loads



Trucks must be tarped & insulated according to the spec:

- Use it to help maintain temperature
- Protect against the elements



Proper Truck Loading Techniques



- Spot check trucks to make sure beds are clean before loading
- Cross-contamination
- Cold material will break loose and segregate mix



Truck-End Segregation



Load-to-load segregation

- Caused by segregation in the truck transferred through the paver.

Make sure the trucks are being loaded in multiple drops!

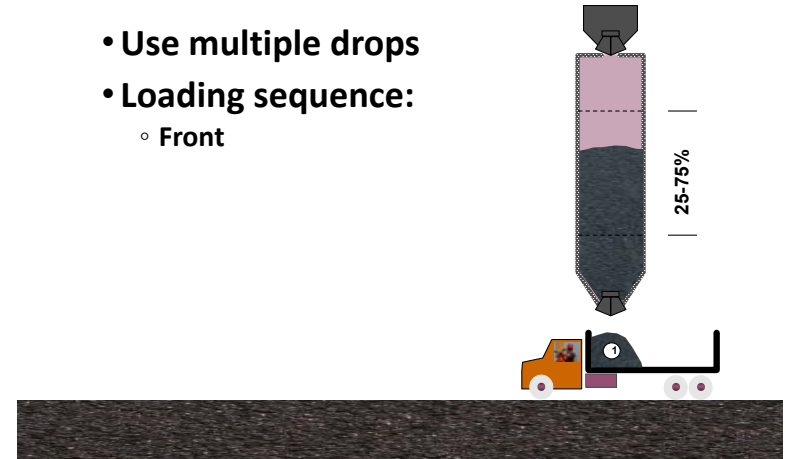
Truck-End Segregation



Truck Loading



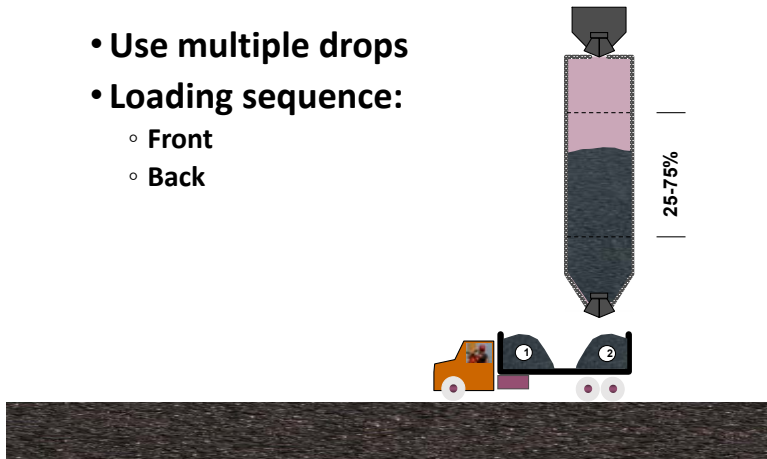
- Use multiple drops
- Loading sequence:
 - Front



Truck Loading



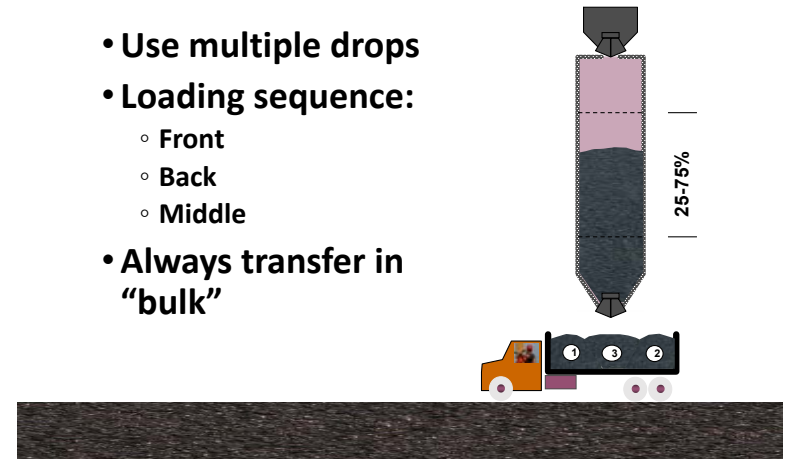
- Use multiple drops
- Loading sequence:
 - Front
 - Back



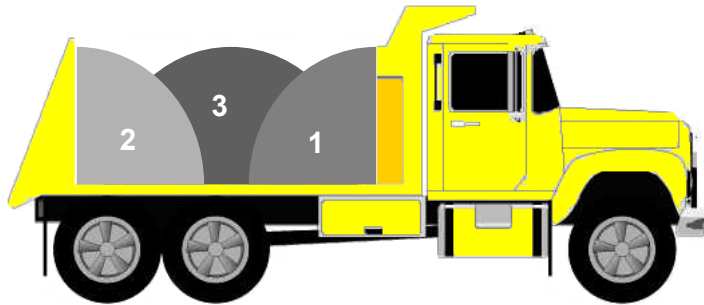
Truck Loading



- Use multiple drops
- Loading sequence:
 - Front
 - Back
 - Middle
- Always transfer in "bulk"



Proper Truck Loading Techniques



Make sure end dump trucks are loaded in this manner to help prevent segregation in the truck.

Proper Truck Loading Techniques



- Semi-trailers more drops help prevent segregation
- Don't allow the mix to be dribbled-in
 - Ensure quick dumps in mass



Unloading the Truck



- Direct feed to paver
 - End dump trailers
- Material Transfer Vehicle



Direct Feed to Paver



- End Dump Trailer
 - Tarps removed off the load
 - Raise bed to break load (material slumps against tailgate)
 - Paver engages truck
 - Release tailgate and continue raising bed so material moves in mass
 - Do not chain (restrict) tailgate opening
 - No trickling mix into paver
 - Paver pushes truck

Improperly Backing a Truck



Improperly Backing a Truck



Loading the Hopper



Break the load before opening the tailgate.
Move the mix in a mass - no trickling!

Material Transfer Vehicles



MTVs are intended to

- Help the paving train keep moving continuously
- Expedite truck unloading and return trips
 - Use will typically reduce two trucks
- Eliminates truck/paver contact
- Reduce potential for:
 - Physical segregation
 - Thermal segregation



Material Transfer Vehicles



- To avoid stop-and-go paving, coordinate:
 - Number of haul units
 - Paver speed
 - Plant production rate
 - MTV speed
- MTVs should continuously remix
- Use caution when crossing bridges



Material Transfer Vehicles



- Specialty uses:
- Offset paving
 - Elevation differences
 - Stringlines for grade control
 - Superelevated curves
 - Small, odd shaped areas



Material Transfer Vehicles



- Hopper inserts are used with MTVs or windrow pickup machines
- They increase the hopper capacity by 50% to 75%
- They help reduce mix segregation



Delivery Checklist



- Use proper release agents
- Good loadout practices
- Proper, tarping, insulation, & hauling
- Proper unloading and dumpman operation
- Pay attention to temperature
- Have enough trucks for a continuous paving operation

Module 4 Placement

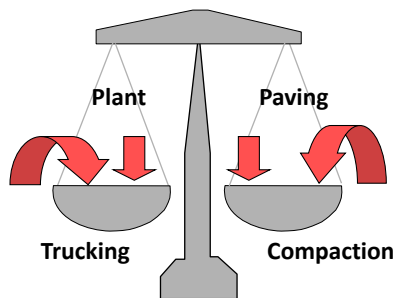
Outline

Placement Best Practices

- Project planning
- Understanding the paver
- Factors affecting the screed
- Screed Adjustments
- Paving process



Project Planning



Balance:

- Project Tonnage
- Hot plant output
- Length of haul
- Traffic conditions
- Number of trucks

Consistency = Quality

Project Planning



Wheeled Pavers

Easy to operate,
inexpensive to
maintain



Tracked Pavers

Better for Agg surfaces
Better traction
More stable platform

Project Planning



Paving Width

- Screed extensions
 - Variable
 - Fixed or rigid
- Auger extensions
- Retaining plates
 - Tunnels

Project Planning



Specifications

- Grade requirements
 - Match existing
- Leveling Sensors
 - Joint matcher
 - Traveling Grade control
 - Position of sensors
- Cross Slope

Project Planning



First Pass Must Be Straight!

- String-lines
- Other reference



Project Planning



Grade Conditions

- Patching
- Bumps
- Low spots
- Milling
- Leveling course
- Transitions

Understanding the Paver



Basic Functions

- Tractor/ power unit
- Material feed
- Self-leveling

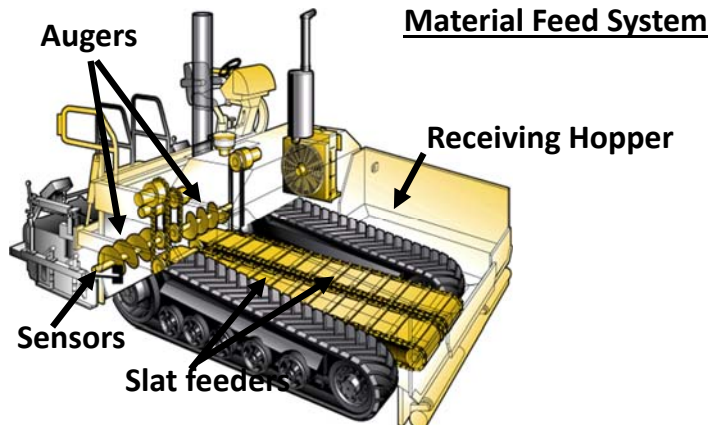
Understanding the Paver



Tractor Self-Leveling

- Power Unit
- Screed can rise & fall
 - Free Floating
- Constant line of pull when set up properly
- Smooth's irregular grade

Understanding the Paver



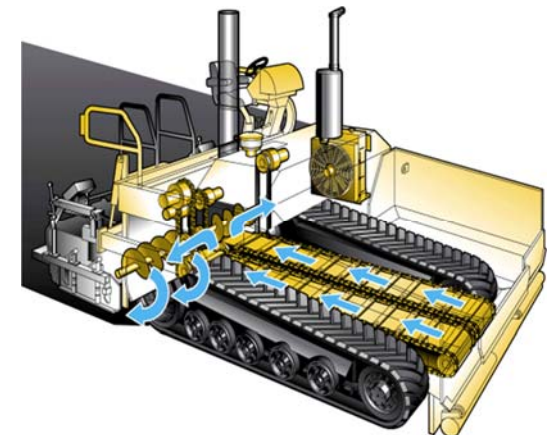
Understanding the Paver



Material Feed System

Flow of Material

- Hopper
- Through tunnel
- In front of screed
- Augers move mix transversely



Visual Inspection of Mix



Problem Indicators



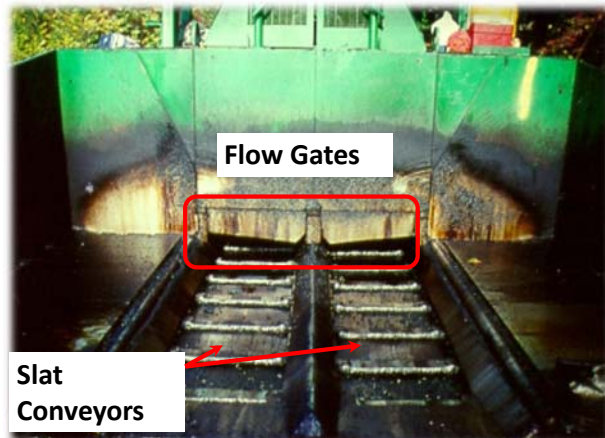
- Blue smoke
- Stiff (high peak)
- Slumped
- Dry, dull appearance
- Moisture
 - Steam
 - Condensate
- Segregation
- Contamination
 - Solid
 - Fuel or solvents

Break the Load



- Move mix in a mass
- Trickling causes segregation

Slat Conveyors and Flow Gates



Hopper Management



Never expose conveyor or fold in hopper wings:

- Mix will be segregated
- Cold mix will be rolled on top of hot mix
- Augers are starved
- Remove corner built-up at end of each paving shift
- If paver stops, leave hopper 1/3 to 1/2 full

Understanding the Paver



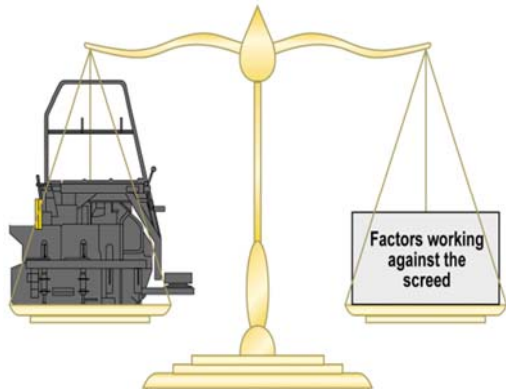
- Keep in good condition
- Scheduled inspection
- Maintenance guidelines
- Safety Guidelines
 - Lockout/Tag out
 - Locking Pins
 - Wheel Chokes



Basic Principle Has Not Changed



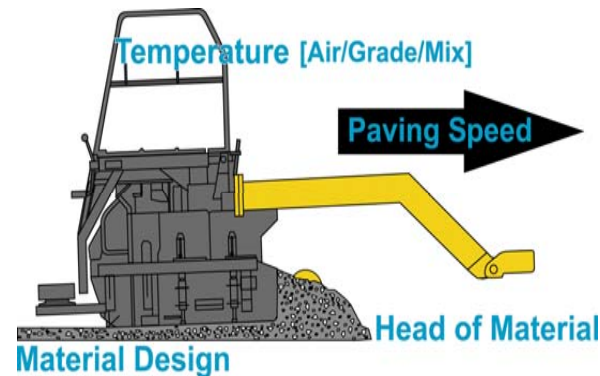
Understanding the Paver



Free-Floating Screed

- *Position* determines mat thickness
- Screed *position*
 - Will remain constant
 - *If all factors* remain constant

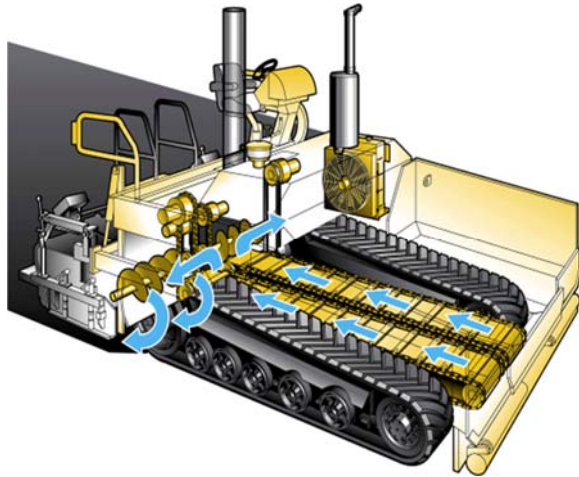
Understanding the Paver



Factors Affecting Screed

- *Head of material*
- Paving speed
- Screed adjustments
- Mix design
- Temperatures
 - Air
 - Grade
 - Mix

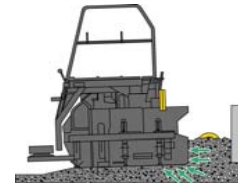
Factors Affecting Screed



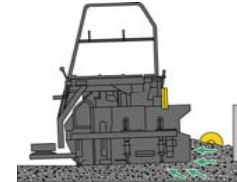
Head of Material

- Uniform flow
- Uniform force against face of screed

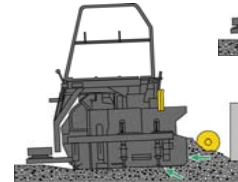
Factors Affecting Screed



- Too much material, screed forced to rise

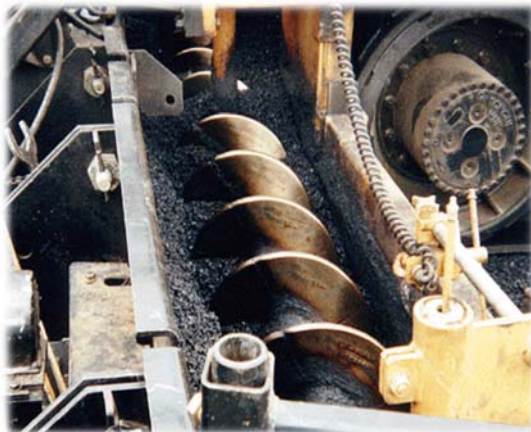


- Correct amount of material, screed remains level



- Too little material, screed will dip down

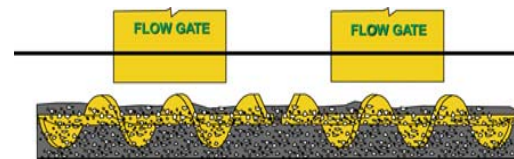
Factors Affecting Screed



Uniform Head of Material

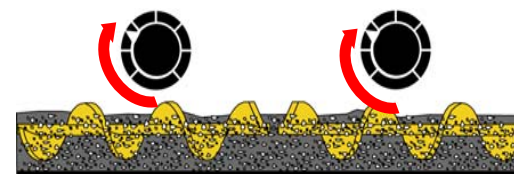
Across Width of Auger

Factors Affecting Screed



Flow Gates Set Properly

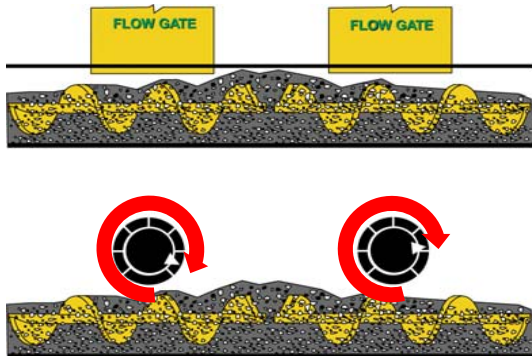
- Material
 - Uniform Amount
 - Shafts half covered



Feeder Ratio Set Properly

- Same principle as flow

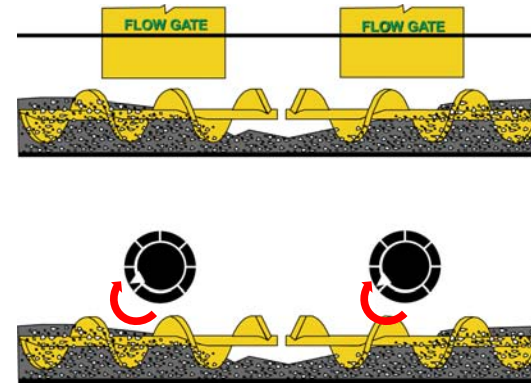
Factors Affecting Screed



Gates/Ratio Too High

- Too much in center
- Screed rises
- Depth increases

Factors Affecting Screed



Gates/Ratio Too Low

- Too little in center
- Screed falls
- Depth decreases

Factors Affecting Screed

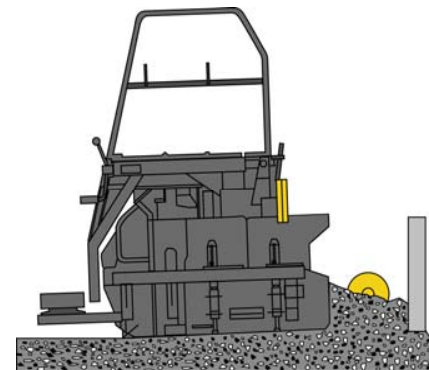


Reverse augers & paddles

- Forces mix under gearbox
- ✓ Simply raising the augers may help prevent centerline segregation
- ✓ Paddles on each auger are in place & in good condition



Factors Affecting Screed



Auger Speed

- Auger speed uniform
- 20-40 rpm
- Too high or too low
 - Cause mat streaks

Factors Affecting Screed



Feed Sensors

- Controls head of material
 - Speed of conveyor & auger
- Situated at end of auger
- Contact or a non-contact
 - Paddle or mercury switch
 - Infrared or ultrasonic sensors

Factors Affecting Screed



No Auger Extensions or Confinement

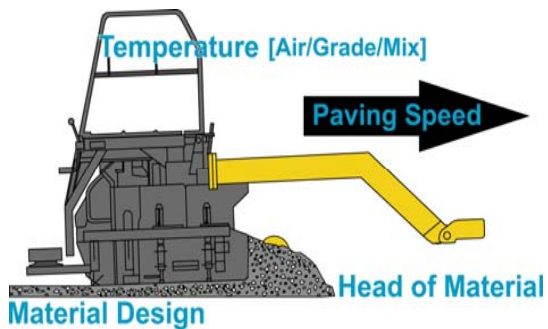
Auger Confinement Tunnels

- Controls mix flow to end of the Screed



Auger Extensions & Confinement

Understanding the Paver



Factors Affecting Screed

- Head of material
- Paving speed
- Screed adjustments
- Mix design
- Temperatures
 - Mix
 - Air
 - Grade

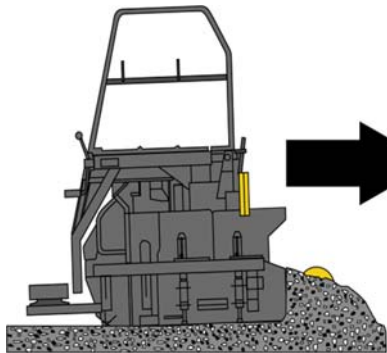
Factors Affecting Screed



Paving Speed

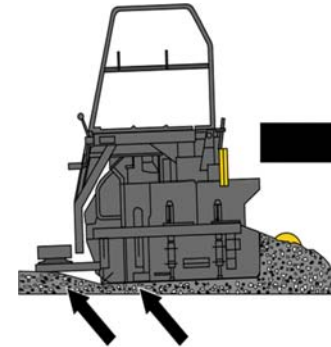
- Constant as possible
- Feeders match
 - Paving speed
 - Speed changes
- Feeders adjusted

Factors Affecting Screed



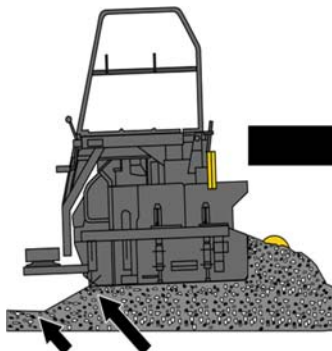
- Constant Speed**
- Shear factor is constant
 - Depth remains constant

Factors Affecting Screed



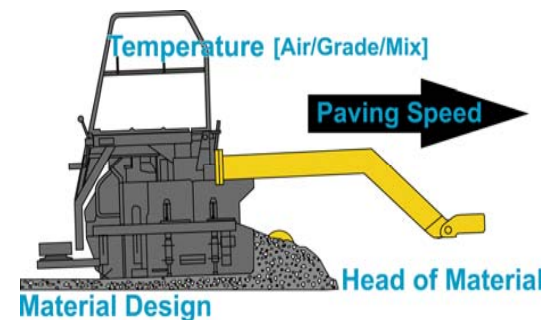
- Increased Speed**
- Shear force decrease
 - Depth decreases

Factors Affecting Screed



- Decreased Speed**
- Shear force increases
 - Depth increases
 - Depth change varies with speed change
 - Type of Mix affects shear force

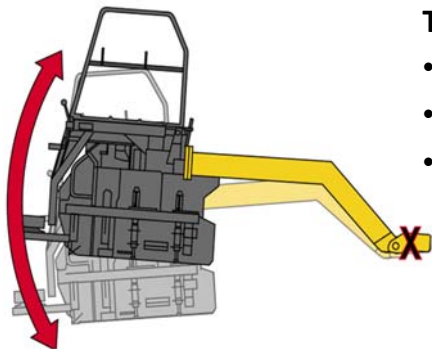
Understanding the Paver



Factors Affecting Screed

- Head of material
- Paving speed
- *Screed adjustments*
- Mix design
- Temperatures
 - Mix
 - Air
 - Grade

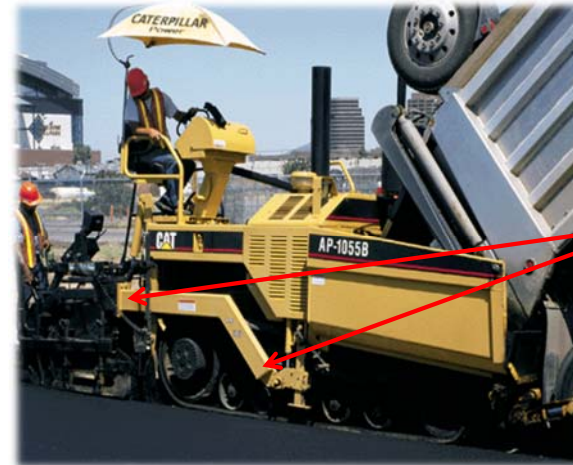
Screed Adjustments



Tow Point

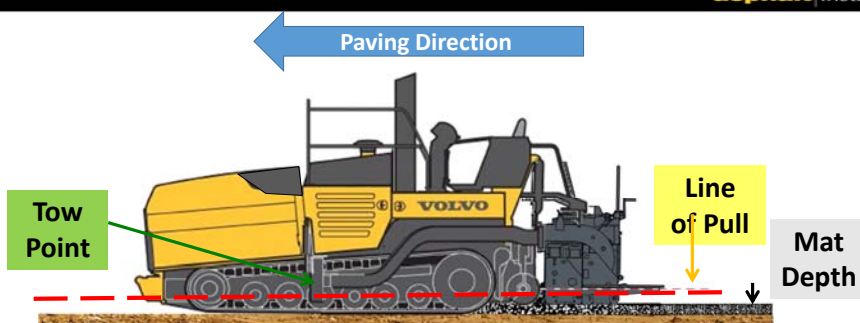
- Fixed on Tractor Unit
- Screed pivots
- Height tow point
- Changes Angle of Attack

Screed Controls



Tow Arms

Line of Pull



Free Floating Screed

- Levels to run parallel
- Imaginary line through the tow points
- Known as 'Line of Pull'

Free Floating Screed Principle



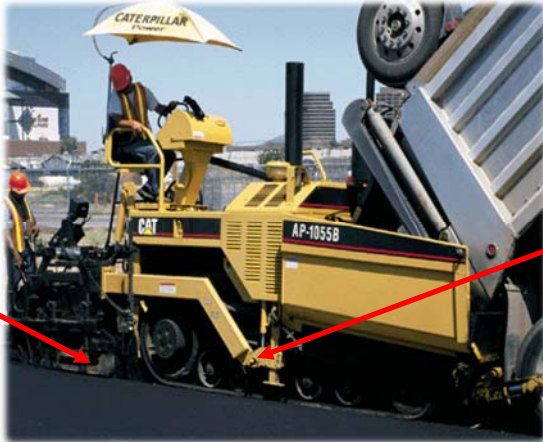
Free floating provides a smooth surface finish by allowing the screed to ride over high spots & fill low spots in the base.

With each lift, smoothness is typically increased around 50%.

Developing the Line of Pull

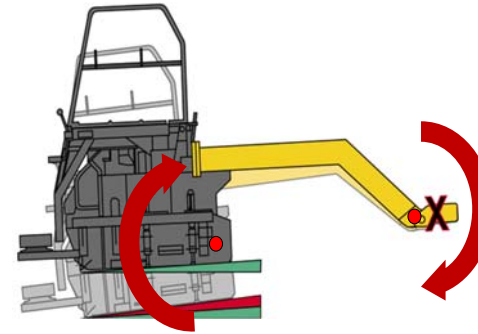


1/8 inch
change



1 inch
change

Screed Adjustments

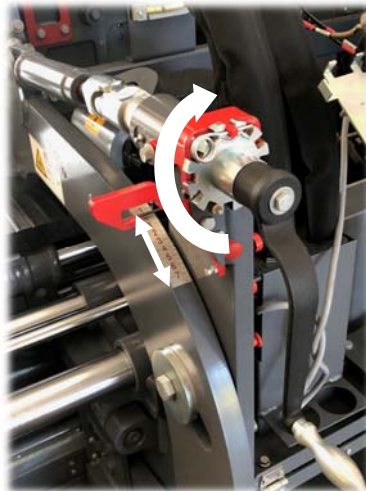


- Pivots on both ends of tow arm
- Angle of Attack

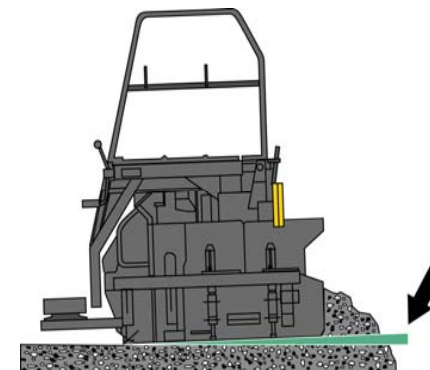
Changing the Angle of Attack



Depth Crank Increases or
Decreases Attack Angle

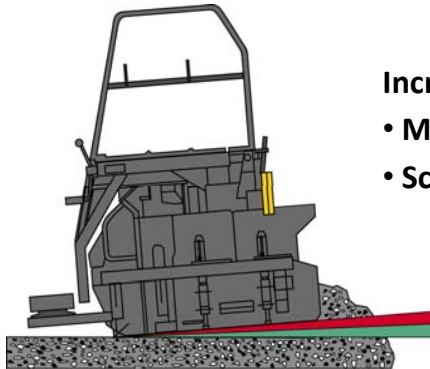


Screed Adjustments



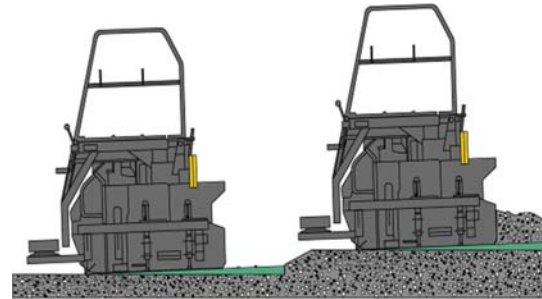
- Starting Angle of Attack
- Screed nose & grade
 - Nose up attitude
 - Screed in equilibrium
 - Free Floating

Screed Adjustments



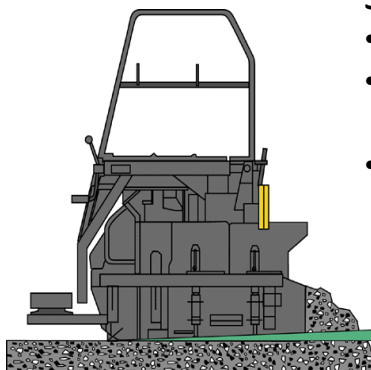
- Increase Angle of Attack**
- More matl under screed
 - Screed rises to new level

Screed Adjustments



- Increased Angle of Attack:**
- Screed climbs
 - Till forces balance
 - Achieves equilibrium
 - Returns to original angle

Screed Adjustments



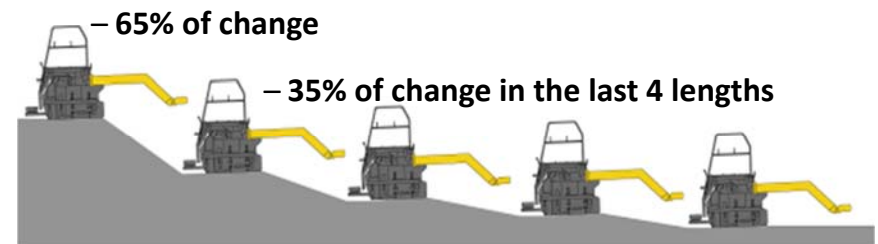
- Screed Angle of Attack**
- Normally 1/8" to 1/4"
 - Too high
 - Wear on trailing edge
 - Too low
 - Increases wear on the nose

↓ 1/8" to 1/4"
↑

Screed Adjustments



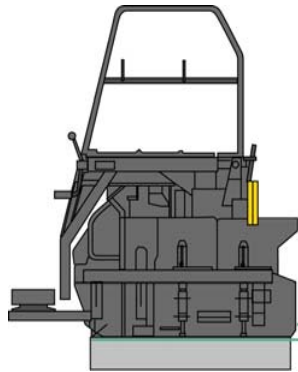
Reaction to Angle of Attack Changes



Takes over 5 tow arm lengths

- Longer tow arm equals a longer distance
 - Improves rideability

Screed Adjustments

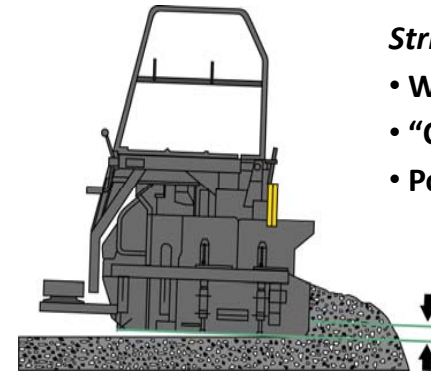


Front Strike-off

- Affects angle of attack
- Factory setting covers most mix designs
- Coarse mix set higher
- Sandy mix set lower

1 inch

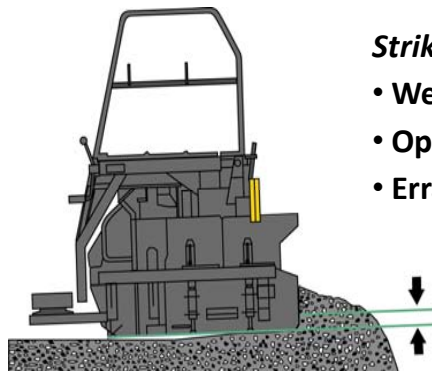
Screed Adjustments



Strike-Off Set Too High

- Wear on nose
- “Chatter”
- Poor mat texture

Screed Adjustments



Strike-Off Set Too Low

- Wear on trailing edge
- Open texture in mat
- Erratic screed behavior

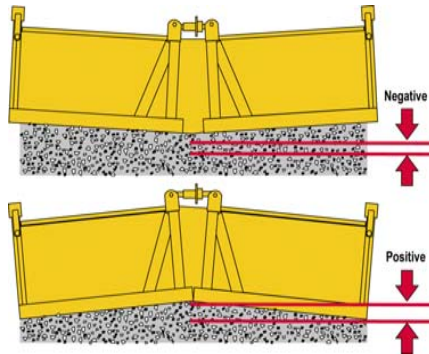
Transverse Slope Control



Uses an internal pendulum

- Sets a reference level
- Dial in cross slope on control side (usually low side)
- Tow cylinders set different angles of attack

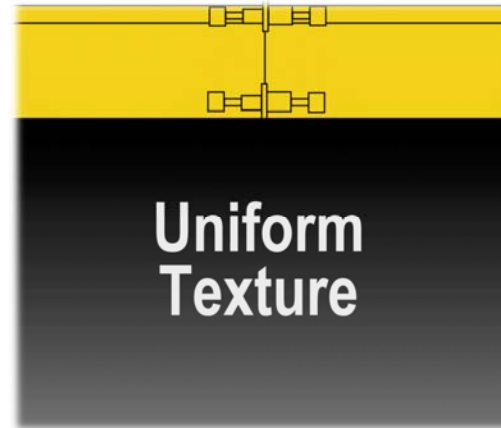
Screed Adjustments



Main Screed Crown

- “Broken”
- Positive crown
- Negative crown

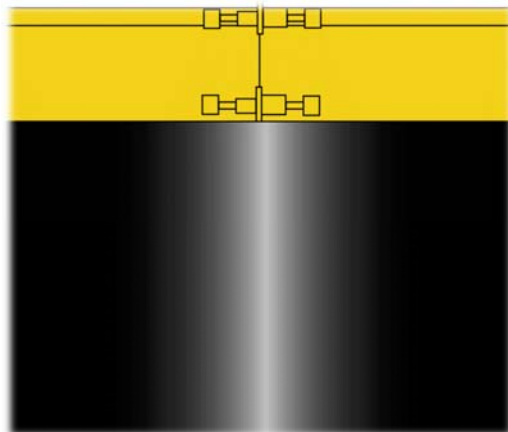
Screed Adjustments



Lead Crown Correct

- Even texture across full width
- Zero crown for most mixes

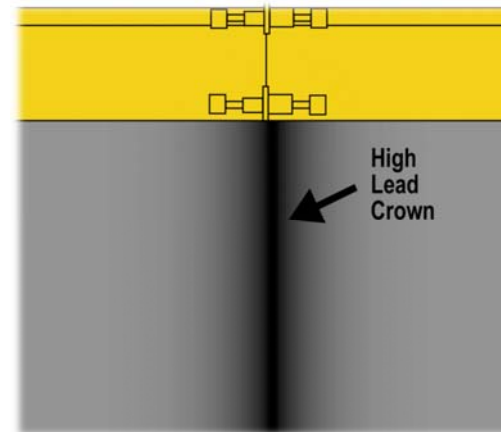
Screed Adjustments



Lead Crown Low

- Open texture in center
- Tight on sides
- Put in 1/8" crown

Screed Adjustments



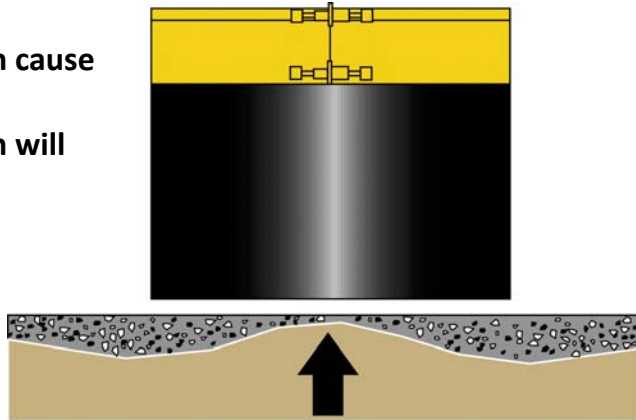
Lead Crown High

- Tight, shiny strip in center
- Open textured sides
- Reduce lead crown

Mat Defects



- Grade conditions can cause texture stripping
- Adjusting lead crown will not correct problem



Due to High Point in Grade

Mat Defects



- Know grade conditions
- High points can cause screed to break aggregates



Mat Defects



- Grade conditions affect quality



Mat Defects



- Oversized material causes:
 - Bumps
 - Streaks



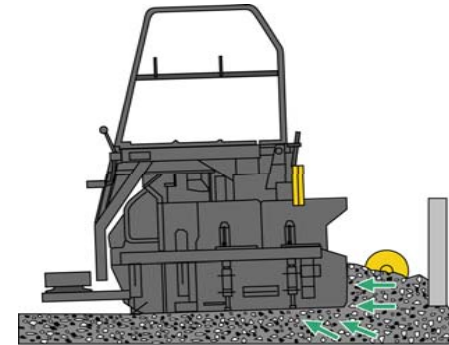
Mat Defects



- Mat thickness should be 2 times greater than maximum aggregate size
- Tendency to fracture aggregate
- Lose control of screed



Understanding the Paver



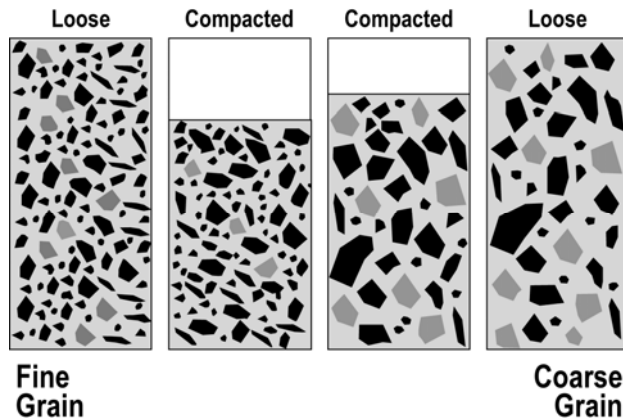
Factors Affecting Screed

- Head of material
- Paving speed
- Screed adjustments
- *Mix design*
 - *Course vs fine*
- Temperatures
 - Mix
 - Air
 - Grade

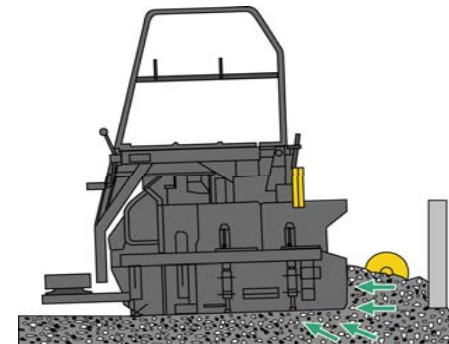
Mat Defects



- Compaction rates vary by mix design
- Adjust laydown thickness to match desired compacted thickness



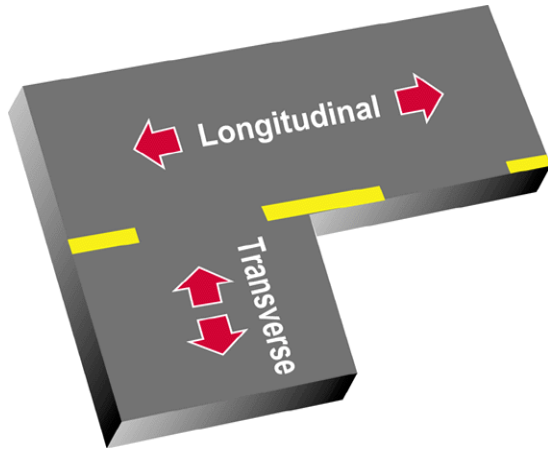
Understanding the Paver



Factors Affecting Screed

- Head of material
- Paving speed
- Screed adjustments
- Mix design
 - Course vs fine
- Temperatures
 - Mix
 - Air
 - Grade

Joint Construction



- Good joints
 - Are no mystery
 - Training & practice
 - Attention to details
 - Miles & miles of L/J

Screed Heating Systems

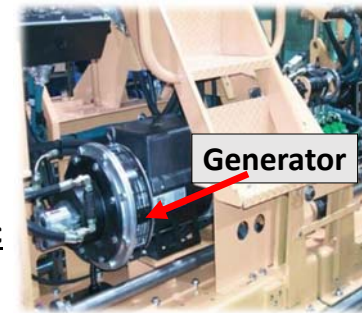


Diesel



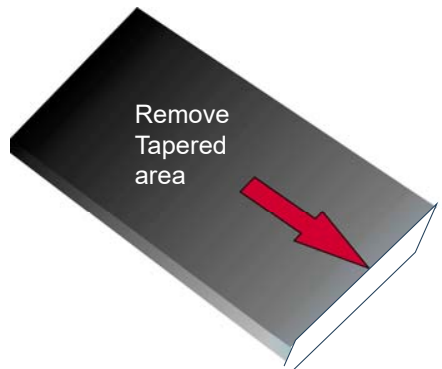
Electric

Propane



Generator

Transverse Joints - Starting



- Pick starting point
 - Hand placed mat
 - Previous day's mat
 - Stop tacking
- Full depth of existing
- Saw cut & remove

Transverse Joints



- Start-up
 - Vertical Edge
 - Clean
 - Tack

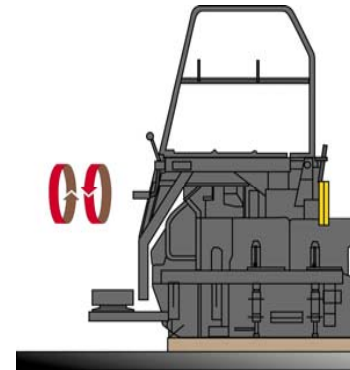
Tack Coat



Full width of mat
Minimizes movement
of unsupported edge



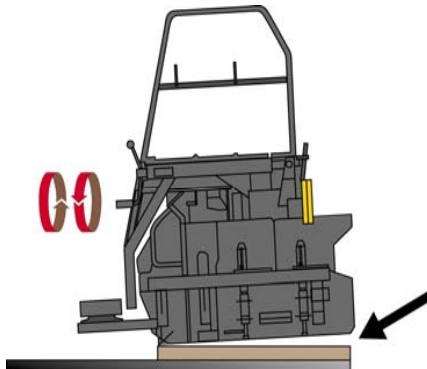
Transverse Joints



- Null screed
- Zero out angle of Attack

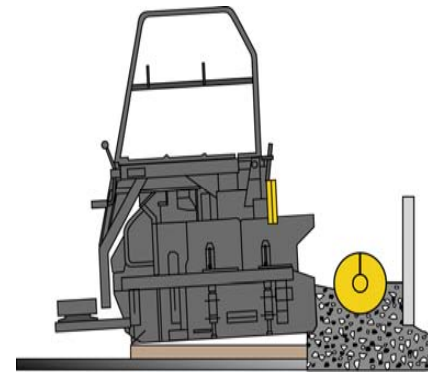
Boards allow for
rolldown thickness

Transverse Joints



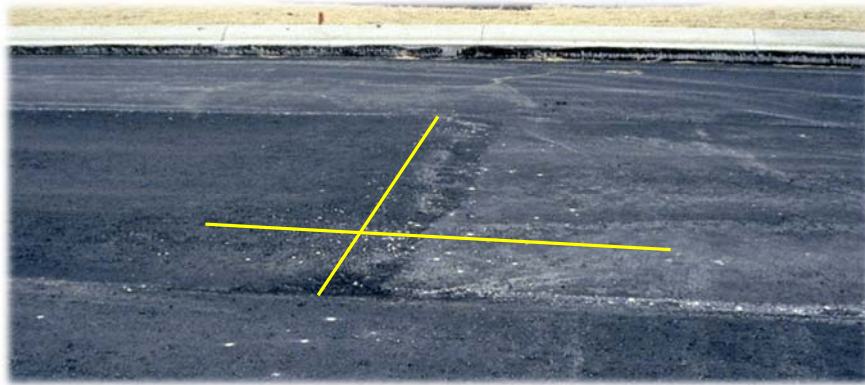
- Introduce angle of attack
- Crank until resistance is felt

Transverse Joints

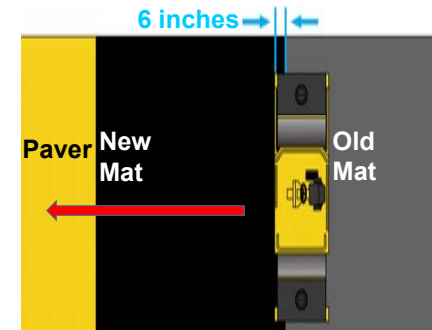


- Fill auger half full
- Conveyor manually
- Auger manually
- Shovel if needed to fill end of screed

Transverse Joints



Transverse Joints



- Roll transverse
- Roll static
- Start on cold side
- Move over in 6" - 8" until on hot side

Transverse Joints

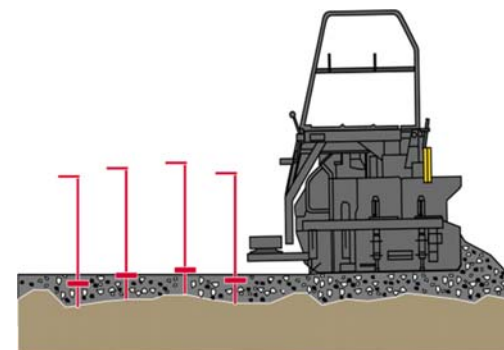


Rolling Transverse Joint

Traffic Safety is always an issue



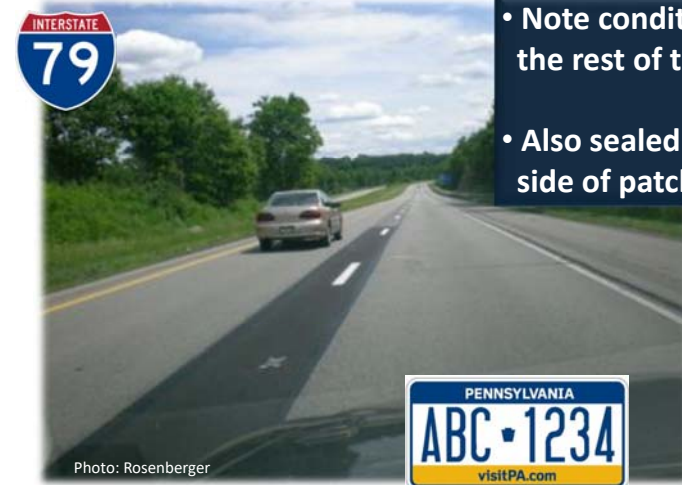
Controlling Yield



Depth Check

- One check, No correction
- Make series of checks
- Average readings
- Correct based on average
- Improves
 - Smoothness
 - Yield

Don't We Already Know How To Build a Longitudinal Joint?



- Note condition of the rest of the mat
- Also sealed each side of patch

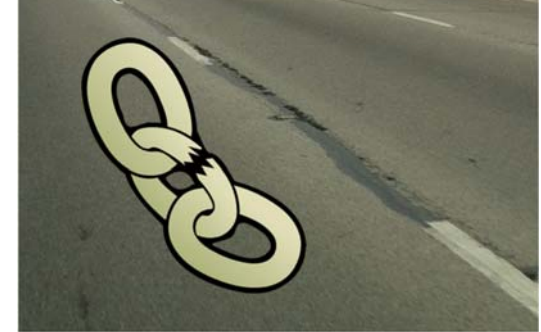


Photo: Grass

I-71 in
Columbus, OH

Too often longitudinal joints are the weak link in an otherwise long-lasting asphalt pavement.

- Agency and industry concern!
- Offers greatest opportunity to improve overall life.



Deteriorated Joint





An Agency and Industry Concern

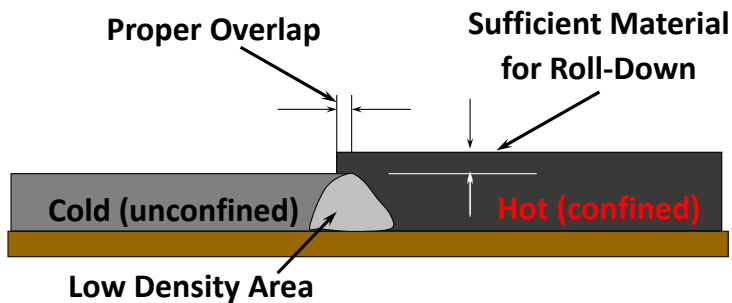
Longevity matters, it impacts:

- ☐ DOT Program Costs
- ☐ Industry's Livelihood
 - o LCCA
 - o Alternate Bid Competitiveness
- ☐ Travelling Public
 - o "...Stay Out"

Clarification of Terms

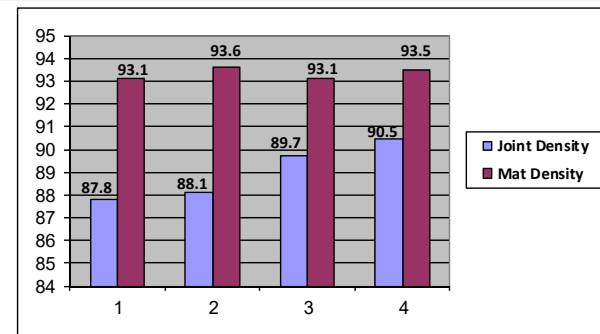
- Density: weight per volume (i.e. 140 pcf)
- Percent Relative Compaction:
 - Comparison of a measured density
 - To a reference density
 - i.e. in place density of 94% TMD
- All industries have jargon
 - Shorthand to simplify communications
- When speaker and slides refer to density, it is jargon for percent relative compaction
 - i.e. 94% density really means 94% TMD

Unsupported Edge Will Have Lower Density



Please note "Cold side" and "Hot side", are the terms used going forward

Joint vs. Mat Density

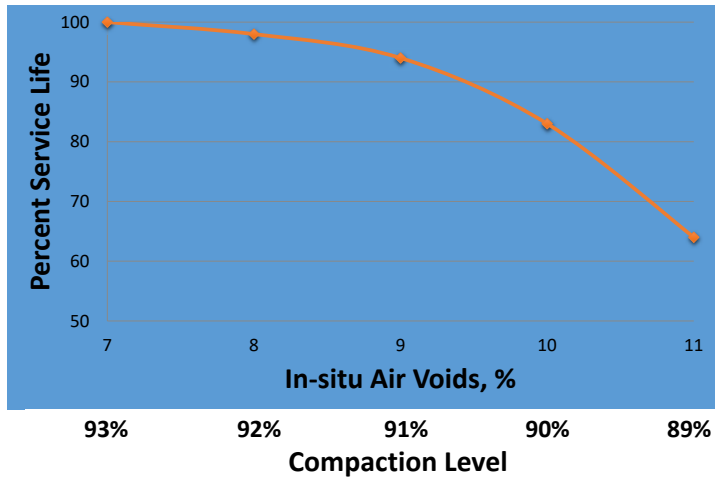


Wearing Surface 12.5mm
Binder Course 19.0mm

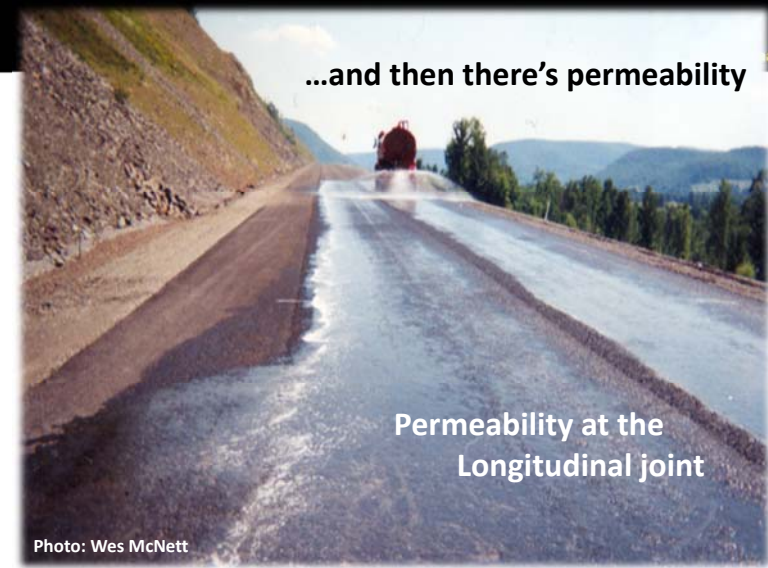


2006-2007, with 6" cores taken over joint

Effect of In-Place Voids on Life Washington State DOT Study



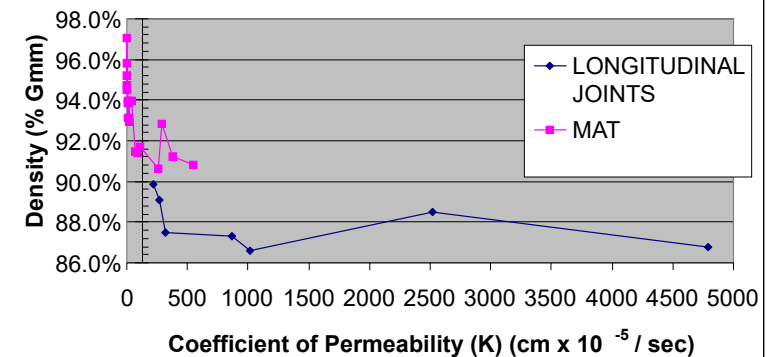
...and then there's permeability

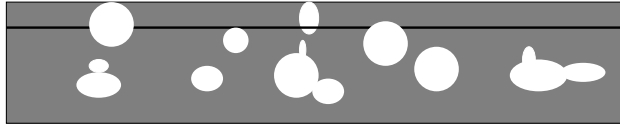


Permeability can be
Catastrophic



DENSITY VS. PERMEABILITY 12.5 mm WEARING COURSE





Air Voids ≤ 7 or 8%

Mix generally not permeable



Air Voids $> 10\%$

Mix generally permeable

Quality Control and Acceptance of Joint Density



Break

Constructing a Quality Longitudinal Joint

- Types of LJs
- Planning for the Joint
- Paving Best Practices (sequentially)

The Best Longitudinal Joint: *Echelon Paving*



New Jersey



Echelon Paving Longitudinal Joint



Joint passes between the quarters

Traffic limits the opportunities to
pave in echelon

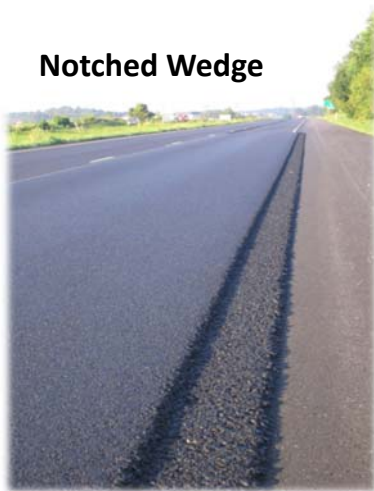


Consequently, most longitudinal joints are
built with a cold joint.

Preferred Joint Type? Experts Divided.



Notched Wedge



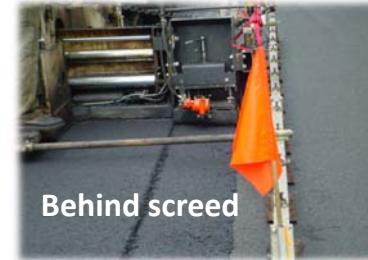
Butt



Front of screed



Behind screed



Device forms & compacts wedge



Plan for Longitudinal Joints...



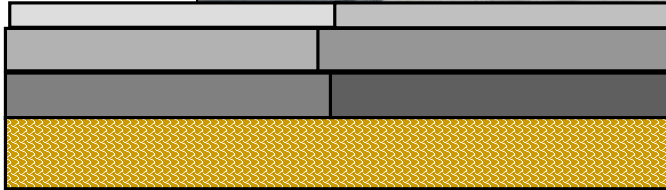
- Joint Type
- Layout Plan of Final Lift showing joints
 - Recognize need to offset joints between layers
 - Avoid wheel paths, RPMs, striping (if possible)
- Testing of Joint
 - Type, location, schedule, by whom
- Joint Construction Practices
 - Paving, rolling, materials
- Pave low to high for *shingle effect*
 - Avoids holding water at joint by hot side being slightly



Plan for Longitudinal Joints...



Offset joints between layers
by at least 6-inches;
surface joint should be near
centerline (not in
wheelpath)



Poor planning –
joint in wheelpath



Avoid Where Striping Will Go



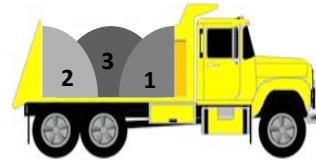
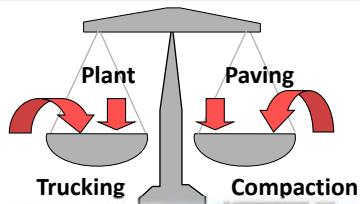
DELDOT

Which Can Result In This



DELDOT

Getting Started Off Right



Dump Person



MTV



Managing mix flow even more critical



MORE IMPORTANT

Over tacking full width of mat



Project Planning



**First Pass
Must Be Straight!**

- String-lines
- Other reference



Great Results

Tough to get proper overlap (1")



Double 55' Ski

Use Automatic
Grade Control
(Versus Manual)

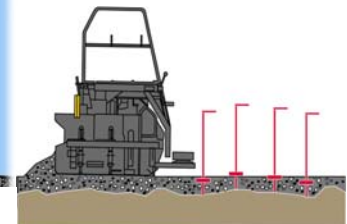


Contact Ski



Ultrasonic Ski

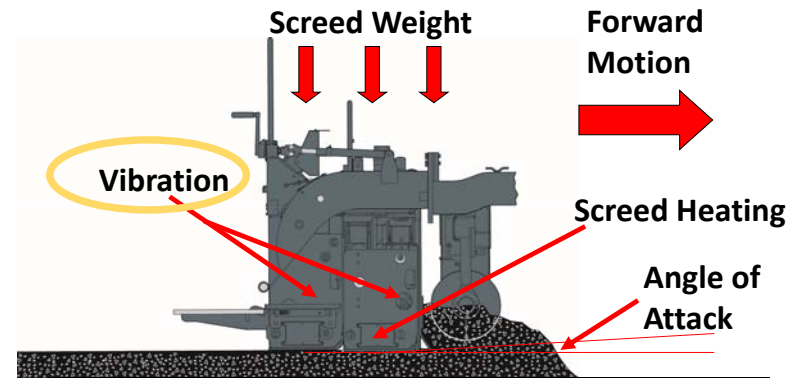
Don't Rely on Manual Windmill Johnny



Vibratory Screed Should Always Be On



Material To Flow & Pre-compaction



END GATE



Seated Flat on the
Existing Surface

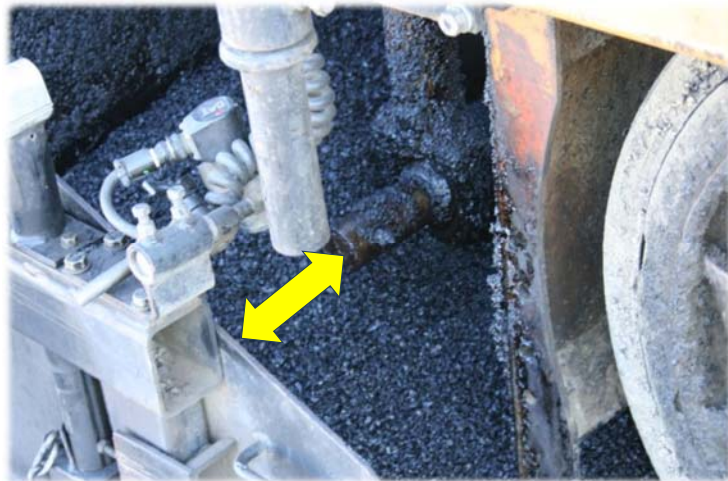


Uniform Head of
Material

Maintained Across
Width of Auger



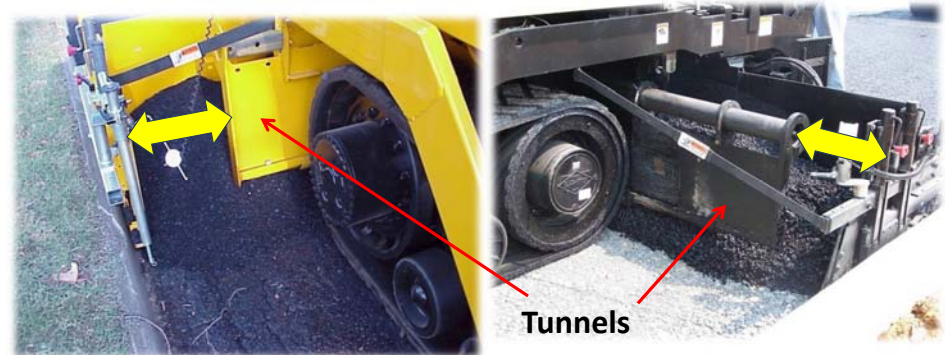
Extend Augers 12-18 Inches Of End Gate



Extend Tunnels the Same Distance



Confine mix to outer edges of screed to deliver homogenous mix at joint



Non-segregated mix at joint:

- Uniform head of mix
- Auger and tunnel extended
- Screed plate running flat



Examples of Auger Overload... Leading to Segregate



Auger Speed



Auger and Tunnel likely not extended within 12 to 18-inches of the end gate

The Result - SEGREGATION at joint



Kept Rollers Close to the Paver



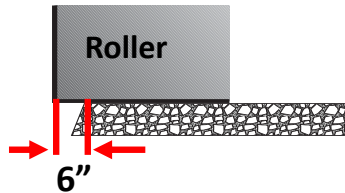
Best Way to Roll a Joint

Rolling Unconfined Side?



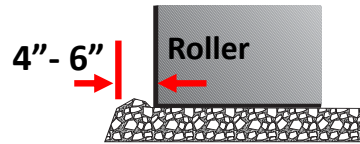
Option #1

Hang over 4-6"

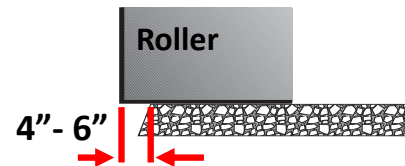


Option #2

1st Pass 4"-6" inside



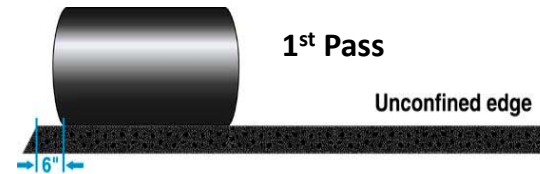
2nd Pass hang over 4"-6"



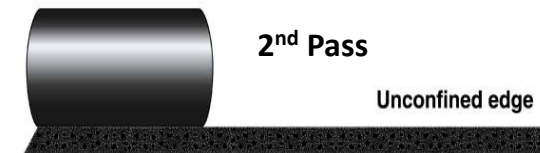
Alternative: Option #2



Stay Back 4-6 inches on 1st pass



- Merit:
 - Minimize lateral movement?

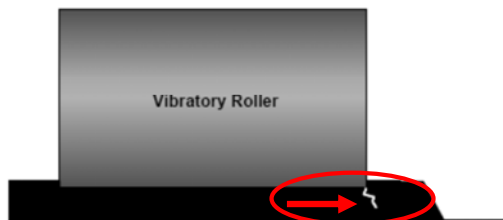


- Concern:
 - Developing stress crack?

What We Don't Want



Rolling Unsupported Edge
With First Roller Pass



Drum passes inside the unsupported edge,
a stress crack can occur

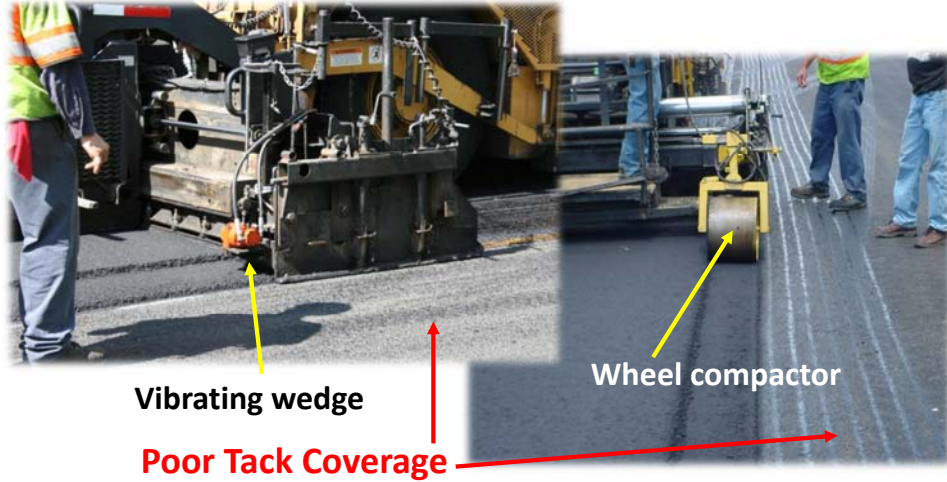
Our Recommendation: Option #1



1st Roller Pass Hangs Over 4-6 inches



Compacting Notched Wedge



Vibrating wedge

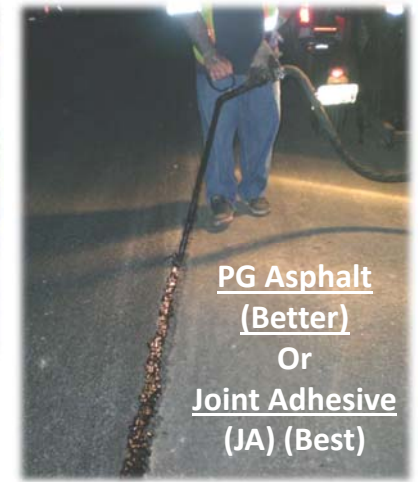
Poor Tack Coverage

Wheel compactor

Sealing Joint (Butt or Wedge)



Emulsion (Good)



PG Asphalt
(Better)
Or
Joint Adhesive
(JA) (Best)

Sealing Joint (Butt or Wedge)



Void Reduction Asphalt Membrane



Joint Adhesive

- Polymer Modified Binder
- Mineral Filler
- Heavy duty distributor

Set Paver Automation



Never Starve the Joint of Material

- Target final height difference of +0.1"
 - NH spec requires 1/8" higher
- Joint Matcher versus Ski
 - Best option to ensure exact thickness
- If hot-side is starved of mix
 - Roller "bridges" onto cold mat
 - No further densification occurs



More Joint Matchers



Ultrasonic Sensor

Optimum sensor location:

- Attached to front of screed



Contact Sensor

Leveling Ski vs Joint Matcher



Setup for smoothness bonus



Note: If underlying pavement:

- Relatively smooth
- Some feel they can use a ski
- Must finish 1/10" high

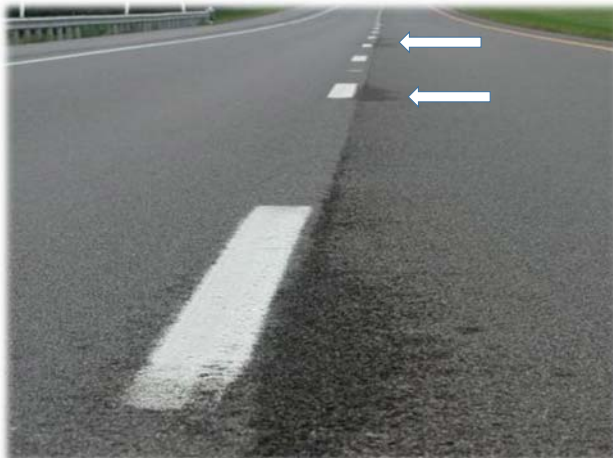
Leveling Ski:

- On cold side for smoothness
- On hot side to insure bonus

Joint Matcher:

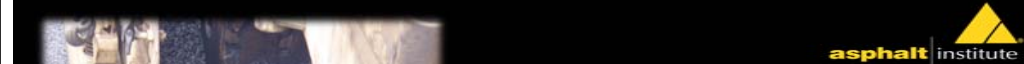
- Used on hot side
- 2nd Pass
- Best for joint

Destined for Failure



Hot side of joint:

- Starved of mix
- Bridging occurred
- Poorly compacted



Proper Overlap:

- 1.0 ± 0.5 inches
- Exception:
 - Milled or
 - Sawed joint
 - Should be 0.5 inches

Do NOT Broadcast Excess Mix



Lute the Longitudinal Joint



Ideal Setup



Rolling the Hot Edge



1st pass all on hot mat
approx 6-8 inches



2nd pass overlaps on
cold mat 3-6 inches

Still Must Watch for Stress Cracks

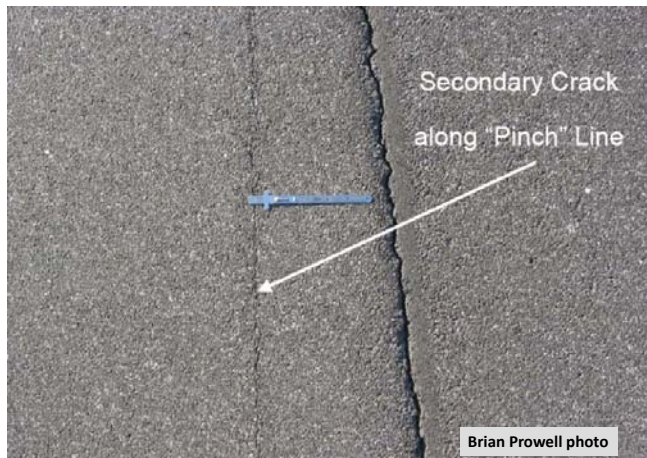


Staying off 6" on 1st Pass



Stress cracks at edge of the drum

Potential Problem with Hot Side "Pinch"



Brian Prowell photo

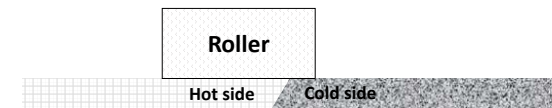
Rolling the Hot Edge



Alternate Method

1st Pass over the confined edge

Roller with edge of drum overhanging
2 to 4-inches on cold side



If insufficient mix on hot side:

- Roller supported by cold mat
- Bridging occurs
- Results in poor compaction



The final pass on a crowned section



HMA Construction Program

Module 5 – Compaction

1

Learning Objectives

1. Objective of compaction
2. Asphalt concrete properties related to compaction
3. Material and mix properties that affect compaction
4. Types of compaction equipment

2

Learning Objectives

5. Selection of compaction equipment
6. Identify compaction variables
7. Main components of compaction equipment maintenance
8. Calculate roller productivity
9. Describe proper compaction operating procedures

3

How to Manage Asphalt Density



BY JESSICA LOMBARDO — JANUARY 29, 2019

- Training
- Temperature
- Proper Roller & Rolling Pattern
- Communication

4

Definitions

- Density
 - the mass of the material that occupies a certain volume
- Compaction
 - the process through which the asphalt mix is compressed and reduced in volume
 - Cannot compress the aggregate or the binder
 - Volume reduction of the mix is the result of squeezing out the air!

5

Definitions

- Pass
 - the entire roller moving over one point in the mat one time
- Coverage
 - the roller moving over the entire width of the mat one time

6

Importance of Compaction

- Improve Mechanical Stability
- Improve Resistance to Permanent Deformation
- Reduce Moisture/Air Penetration
- Improve Fatigue Resistance
- Reduce Low-Temperature Cracking Potential

7

Importance Of Compaction

During the construction of HMA, compaction is considered to be the most important factor that contributes to the performance of the pavement.

“Density is not only a top quality indicator, in terms of how long the pavement will last, but it is also a top pay item in most state specifications,”

rollers are the last piece of equipment to touch the mat after it is placed and are the last opportunity to “undo” the smooth mat that the paver has placed

8

Topics

- Factors affecting compaction
- Time available for compaction
- Roller types
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

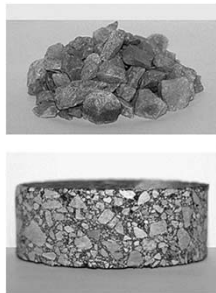
9

Factors Affecting Compaction

- Properties of the Materials
- Environmental Variables
- Laydown Site Conditions

10

Properties of the Materials

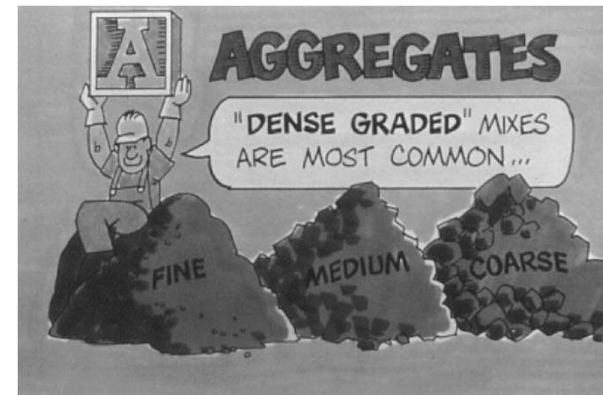


- Aggregate
- Asphalt Binder
- Mix Properties

CAT

Courtesy of Caterpillar Paving Products

11

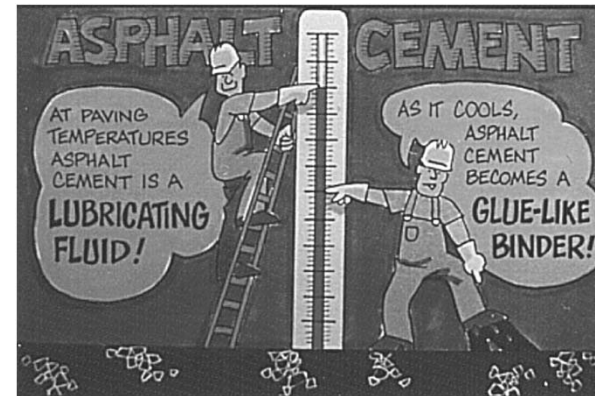


12

Aggregate Carries the Load



13



14

Binder and Compaction

- Asphalt binder holds particles together
 - Provides lubrication at high temperatures
 - Provides cohesion at in-service temperatures
- Prevents air and water intrusion into mat



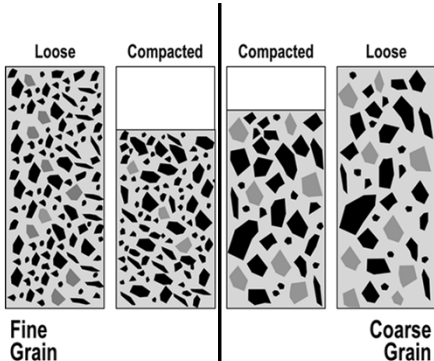
15

HMA



16

Mix Properties



- Compaction rates vary by mix design
- Adjust placement thickness to match desired compacted mat thickness

CAT Courtesy of Caterpillar Paving Products

17

Mat after Compaction



- 4%-8% theoretical air voids allow for binder expansion
- Aggregates moved closer together
- Provides cohesion, impermeability, and stability

CAT Courtesy of Caterpillar Paving Products

18

Topics

- Factors affecting compaction
- Time available for compaction
- Roller types
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

19

Rate of Cooling Variables

- Layer Thickness
- Air Temperature
- Base Temperature
- Mix Laydown Temperature
- Wind Velocity
- Solar Flux



20

Heat = Compaction

- Minimum temperature to achieve final density is 175 F (165 F sometimes)
- In general, an increase of mat thickness by 50% will result in almost twice as much time for compaction
- HMA delivered at 300 F, Air Temp = 40 F, and Surface Temp = 50 F, Clear and Dry, Wind = 5 mph
 - 1.0" cools to 175F in about 8 minutes
 - 1.5" cools to 175F in about 15 minutes

21

DOH SPEC 401.10.4

- The required density shall be obtained prior to the mat reaching a temperature of 175F. The contractor shall be allowed to lower this temperature to 165F if they can demonstrate during the first day of placement of each lift on each project that additional densification can be achieved without causing any pavement distress.

22

PaveCool

- Actual calculation of pavement cooling times based on job site conditions
- Available FREE

<http://www.dot.state.mn.us/app/pavecool/>
 Download PaveCool 3.0 (EXE 6 MB)

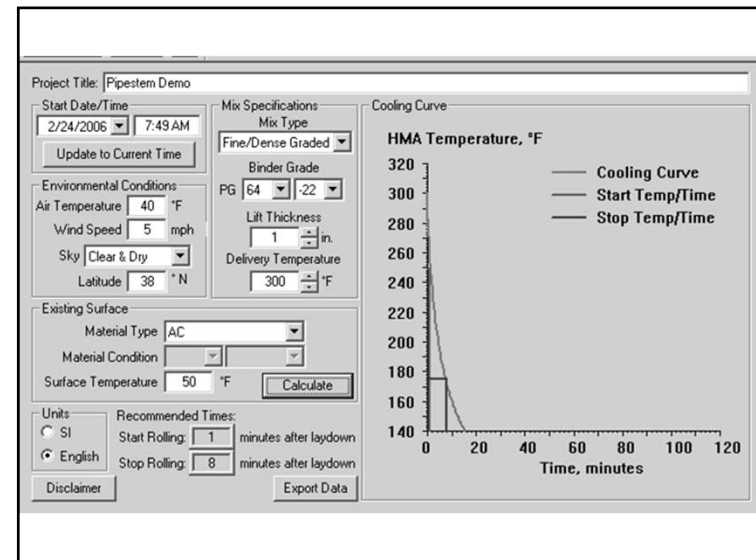
November 2015 (CD available upon request)



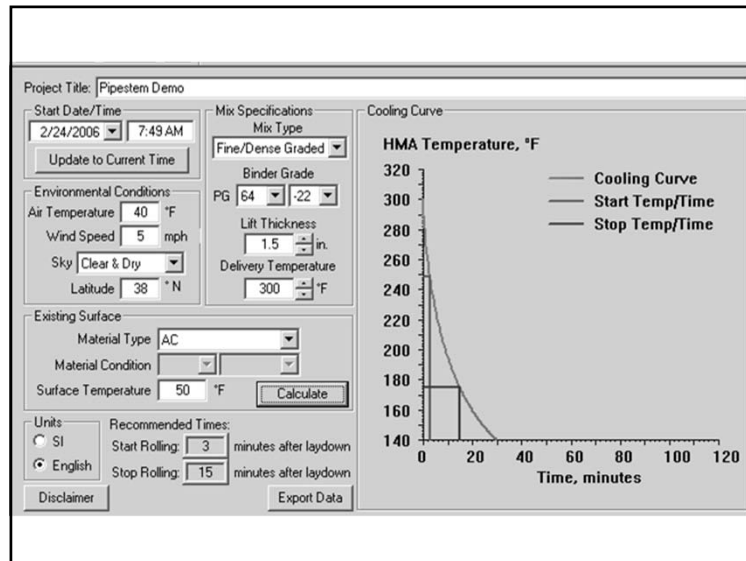
[PaveCool.exe](#) (save this file to your desktop to run PaveCool 3.0 without installing it)

System Requirements
 Windows XP, Vista, 7, 8 or 10
 20 MB disk space

23



24



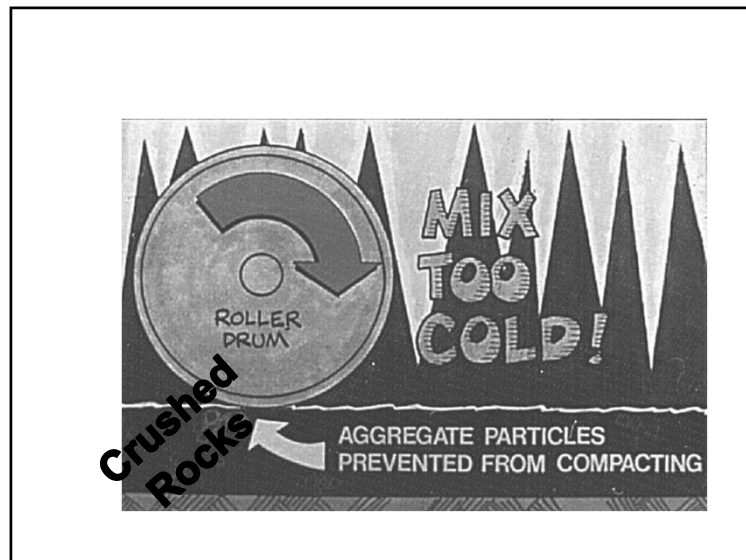
25

Major Factors Affecting Rolling Time

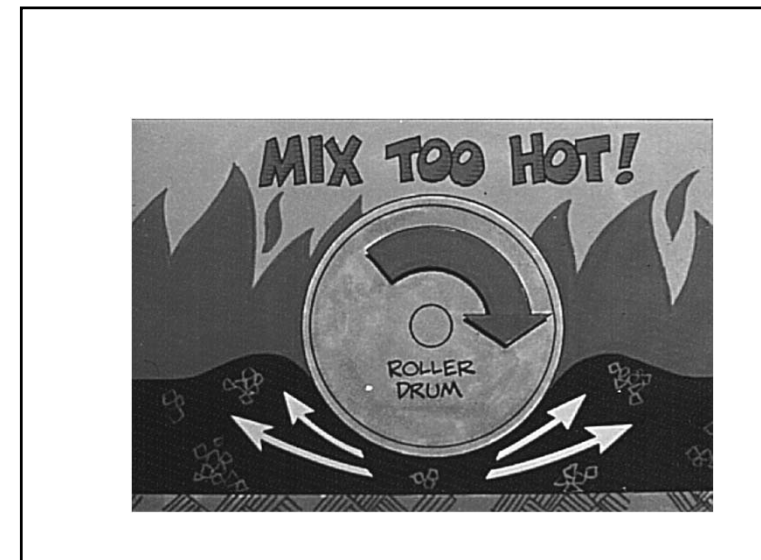
FACTORS	To allow MORE time
Mat Thickness	?
Mix Temperature	?
Base Temperature	?

5-26

26

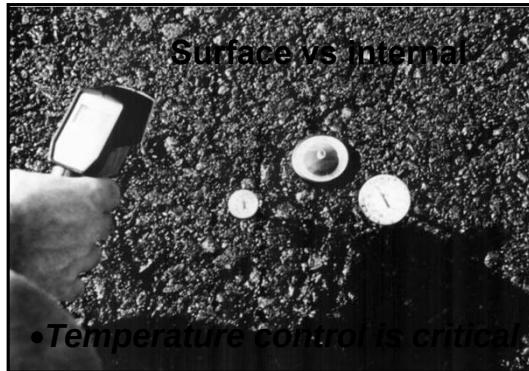


27



28

- Take Regular Mat Temperature Readings

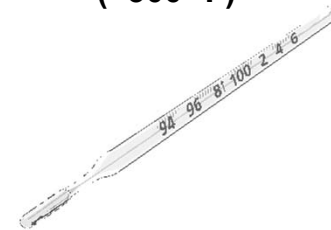


29

Typical Compaction Temperature Range

80 °C
(175 °F)

- 150 °C
(~300 °F)



30

Laydown Site Conditions

- Lift thickness is determined by aggregate size
 - Marshall ~2x maximum aggregate size
 - Superpave ~3x nominal maximum agg. size*
 - Check with design directive 644 for specifics
- Lifts with variable thickness
 - Patch and Leveling
 - Scratch
 - Wedges and other unique conditions

31

MARSHALL VS SUPERPAVE

- | | |
|----------------|-----------------------------|
| • MARSHALL | • SUPERPAVE |
| • Wearing I | • 9.5 mm |
| • NMA 3/8" | • NMA 3/8" |
| • MAS 1/2" | • 3*(3/8) = 1 1/8" ➡ 1 1/4" |
| • 2X(1/2) = 1" | |

32

MARSHALL VS SUPERPAVE

- | | |
|--------------|--------------------|
| • MARSHALL | • SUPERPAVE |
| • Wearing IV | • 19 mm |
| • NMA3 3/4" | • NMA3 3/4" |
| • MAS 1" | • 3*(3/4) = 2 1/4" |
| • 2X(1) = 2" | |

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Laydown Site Conditions

- Base/Existing Surface Conditions
 - Oxidized pavement
 - Rutted pavement
 - Cracked pavement
 - Soft and yielding

34

Topics

- Factors affecting compaction
- Time available for compaction
- Roller types and stages
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

35

Types of Rollers

- Static Steel Wheel
- Pneumatic – Rubber Tired
- Vibratory
- Oscillating



CAT

Courtesy of Caterpillar Paving Products

36

Oscillating Roller I 79 Flatwoods



37

How Do Rollers Compact?

By applying their load
over a given area!

(Contact Pressure)

38

Static Steel Wheel Roller

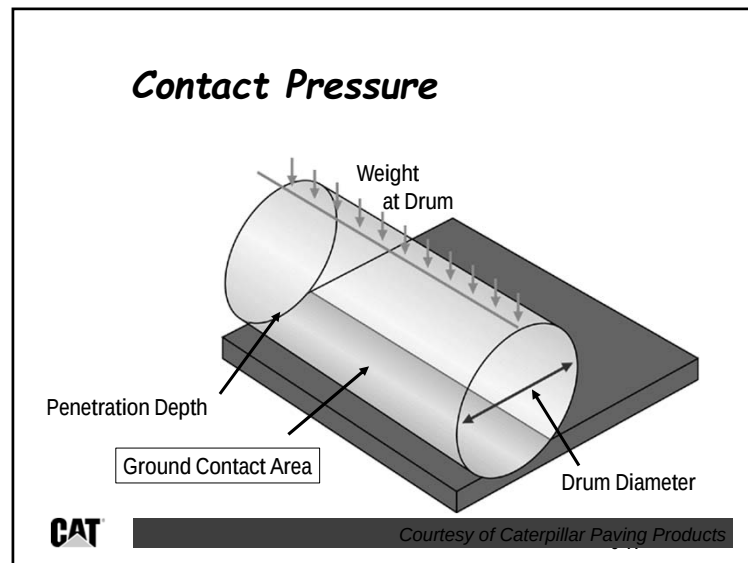
- Contact Pressure
- Operation

39

Static Steel Wheel Roller



40



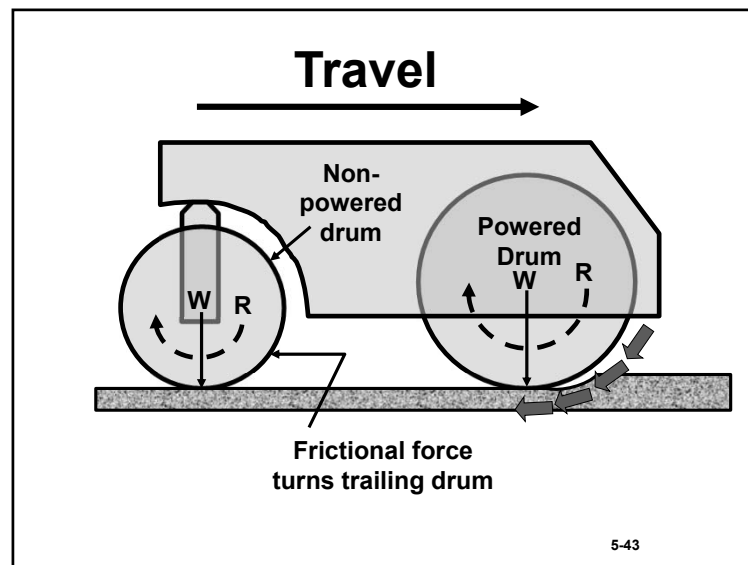
41

Roller Contact Pressure

Roller Contact Pressure at Varying Penetration Depths for 12 ton Static Roller

Penetration Depth (in)	3/4"	1/2"	1/8"	1/16"
Contact Pressure (psi)	36	46	88	132

42



43

- ### Pneumatic Tired Rollers
-
- Wheel load
 - Tire design
 - Inflation pressure
 - Contact area

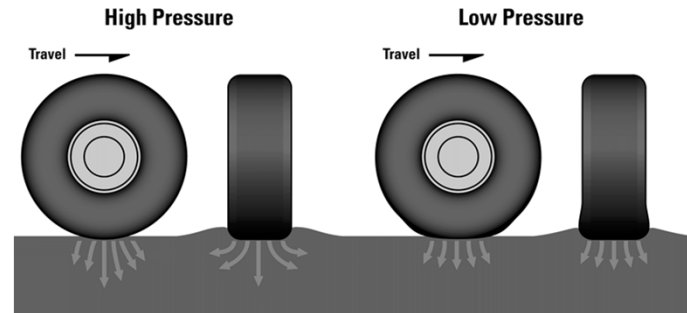
44

Pneumatic



45

Tire Inflation Pressure Versus Ground Contact Pressure



Courtesy of Caterpillar Paving Products

46

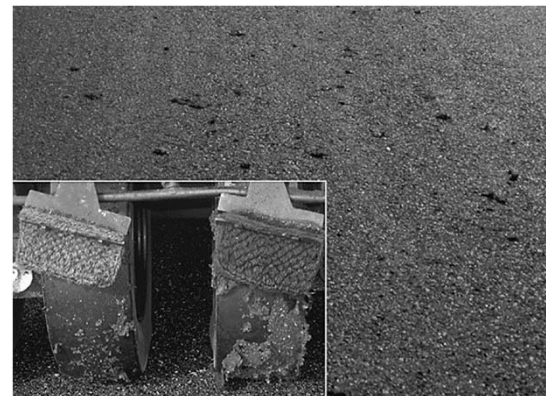
Pneumatic Roller I79 Flatwoods

**Insufficient
heat in the
tires!!!**



47

Tire Pick Up



48

Skirted Pneumatic Roller



49

Pneumatic Roller Operation



“ideal for uneven courses”

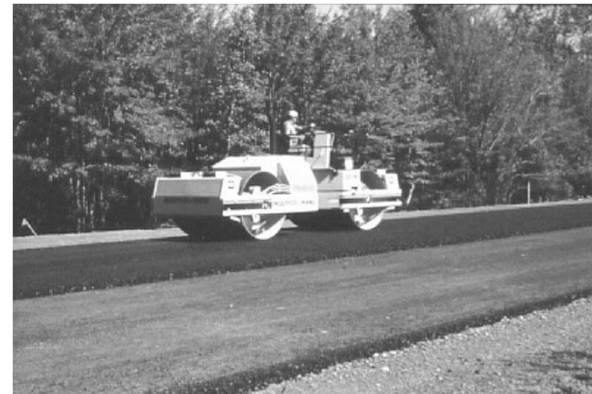
50

Vibratory Roller



51

Single Articulated Frame



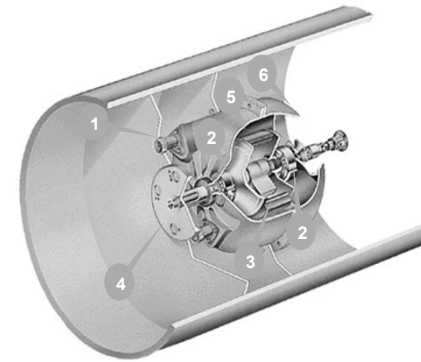
52

Double Articulated Frame



53

Eccentric Weight System



1. Oil level sight gauge
2. Eccentric weight shaft bearings
3. Three-position counterweight
4. Amplitude selection wheel
5. Fixed eccentric weight
6. Pod-style housing

CAT

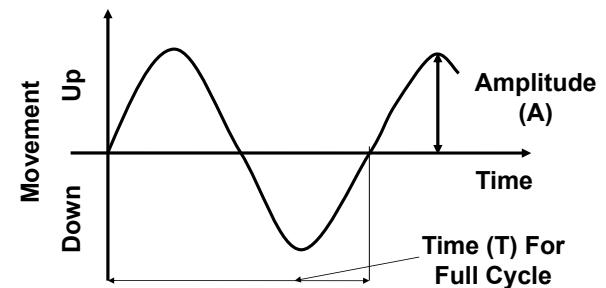
Courtesy of Caterpillar Paving Products

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Vibratory Rollers

- Amplitude
- Frequency
- Impact Spacing
- Operation

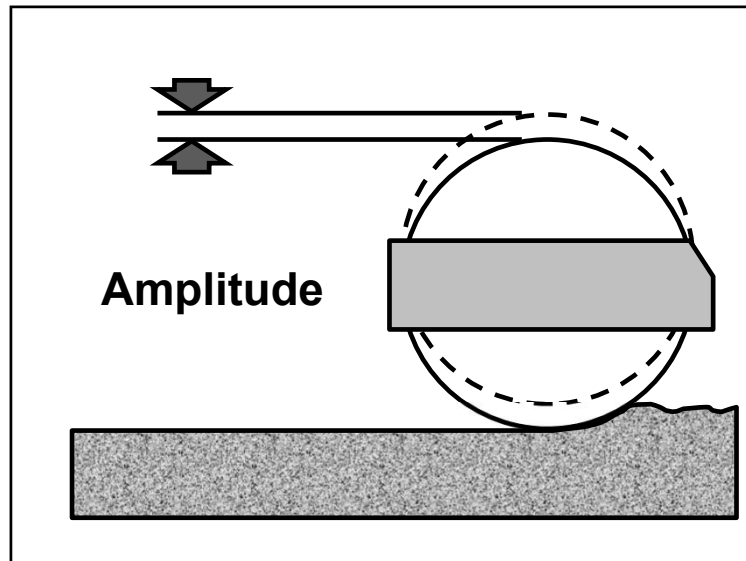
55



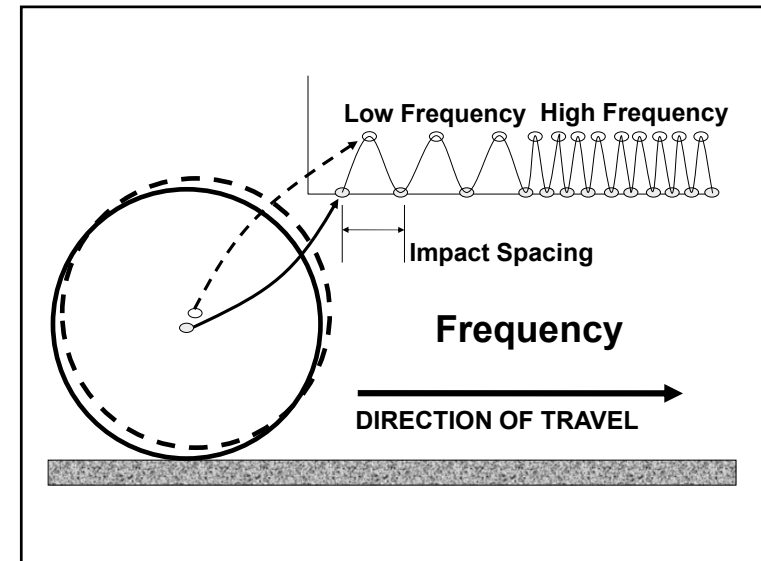
Frequency, f = the number of hertz (cycles/s)--a single cycle is one full rotation of the eccentric weight. Frequency = $1/T$

Amplitude, A = the maximum deviation from position at rest -- one-half the total movement.

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Typical Data for Vibratory Tandem Rollers

<i>Vibratory Steel Tandem ton</i>	<i>Oper. Wt. lb</i>	<i>Drum Diam. ft</i>	<i>Drum Width ft</i>	<i>Static Drum lb/in</i>	<i>Dynamic Drum lb/in</i>	<i>VPM</i>	<i>Nom. Amp. in</i>
6.0-8.0	14,700	3.6	4.6	130	260	2,900	0.025
9.5-11.0	20,500	3.9	5.6	158	384	2,600	0.03
> 13.0	30,000	4.9	6.9	186	423	2,400	0.03

Increase due to vibration

59

Impact Spacing

- Ideal is 10-12 impacts per foot

60

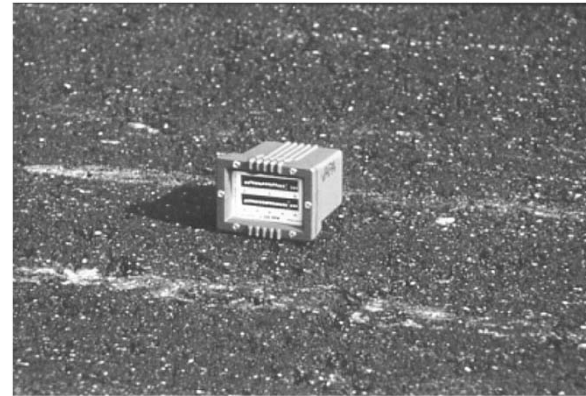
Example

- 2900 Vibrations (impacts) per minute
- 11 impacts/ft
- Determine speed of roller



61

Reed Tachometer



62

Improper Impact Spacing



63

Topics

- Factors affecting compaction
- Time available for compaction
- Roller types
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

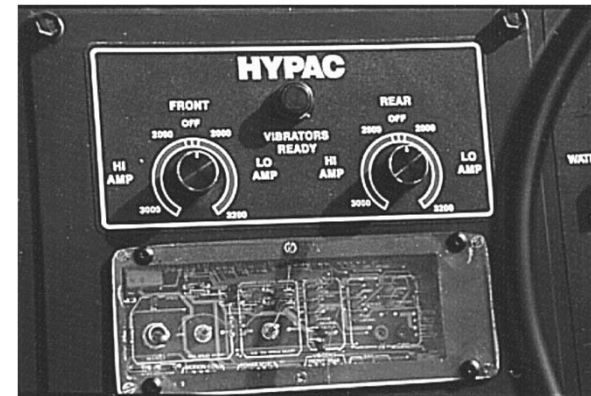
64

Roller Operator Controls

- Speed
- Starts & Stops
- Pattern
- Amplitude
- Frequency
- Distance to paver

65

Roller Controls



66

Compaction Variables

- Roller Speed
- Number of Coverages
 - Pass - the entire roller moving over one point in the mat one time
 - Coverage - the roller moving over the entire width of the mat one time
- Rolling Zone
- Rolling Pattern

67

Stages Of Rolling

- ✓ Breakdown Rolling - Where most of the actual densification is achieved
- ✓ Intermediate Rolling - Where a small amount of additional density is achieved *(needed when breakdown rolling does not provide sufficient density)*
- ✓ Finish Rolling - Used to remove roller marks and finish the surface *(Very little additional densification is achieved)*

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Breakdown Rolling

- Determine the rolling zone by:
 - Experience
 - Estimating
- Should be completed before the surface temperature of the mix falls below 240° F.
- Operated at the highest possible frequency
- Amplitude setting that is dependent on the thickness of the asphalt concrete

<https://www.forconstructionpros.com/asphalt/article/12188306/how-to-compact-asphalt-pavements>

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Intermediate Rolling



Temperature
between 240°F to
190°F
Completion of
density gain.

Vibratory or
pneumatic rollers

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Finish Rolling



- Finish rolling normally takes place within a temperature range of 185°F down to 175°F (165°F).
- Static steel-wheel finish roller
- Marks from other rollers can be removed from the surface of the layer without adding new marks by the finish roller itself.
- Finish rolling for a stable mix is accomplished at higher temperatures than finish rolling for a tender mix.

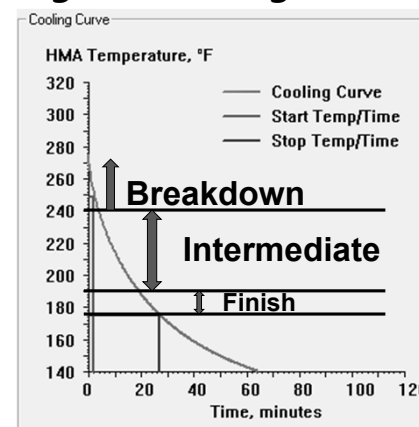
CAT

Courtesy of Caterpillar Paving Products

<https://www.roadbridges.com/compacting-hot-mix-asphalt-pavements-part-ii>

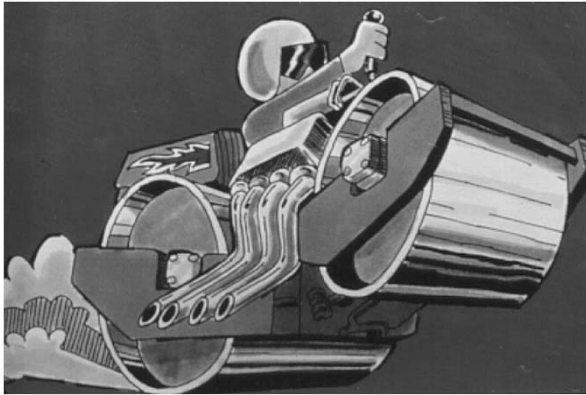
71

Approximate temperatures for stages of rolling



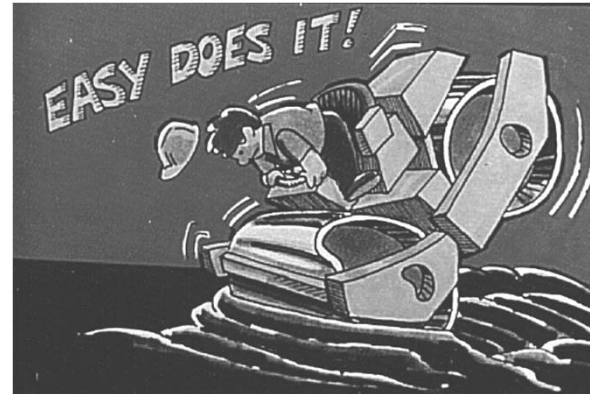
72

Speed



73

Stopping



74

Typical Range of Roller Speeds (mi/hour)

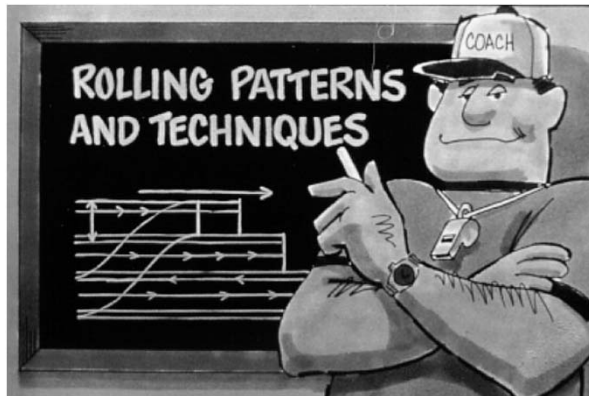
Type of Roller	Breakdown	Intermediate	Finish
Static Steel Wheel	2.0 to 3.5	2.5 to 4	3.0 to 5.0
Pneumatic	2.0 to 3.5	2.5 to 6.4	4.0 to 7.0
Vibratory	2.0 to 3.0	2.5 to 3.5	-----

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Topics

- Factors affecting compaction
- Time available for compaction
- Roller types
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

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Roller Widths



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Courtesy of Caterpillar Paving Products

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Paving Widths



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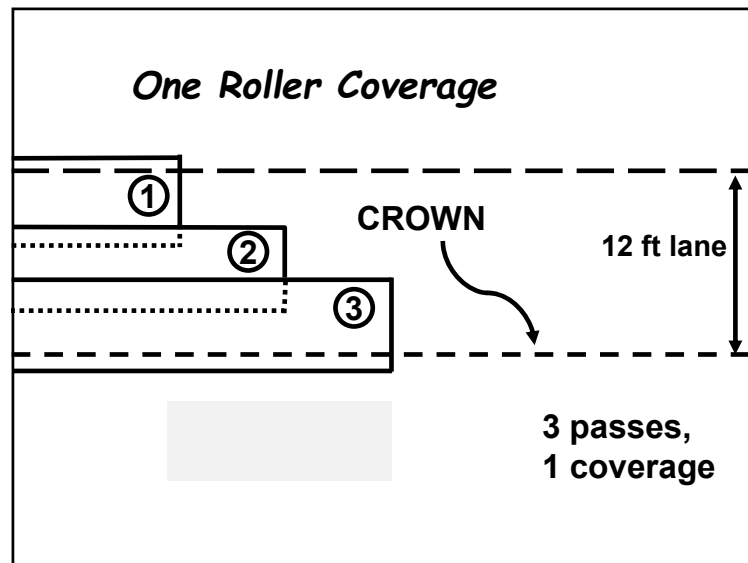
Passes and Coverage

Each time the roller goes over a specific point is ONE PASS.

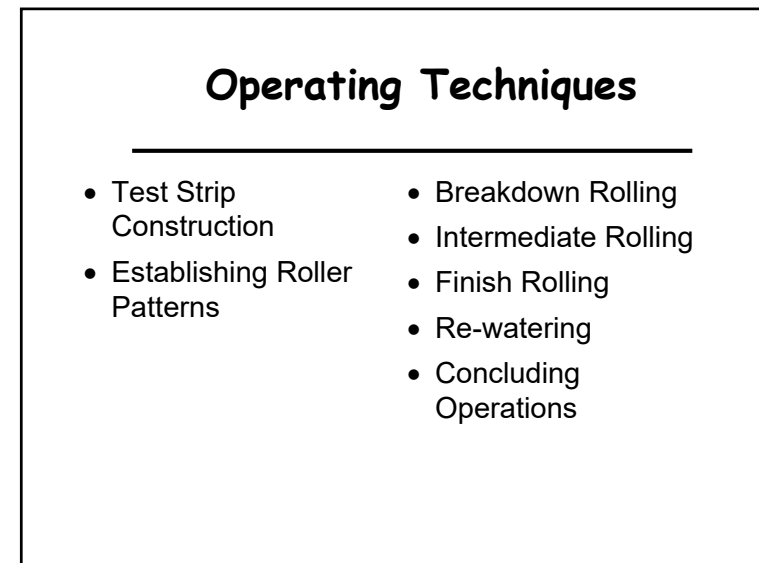
Paving widths are greater than roller width so more than one pass is required to complete a COVERAGE across a pavement.

How many passes of the roller are needed to cover the width of the mat one time ?

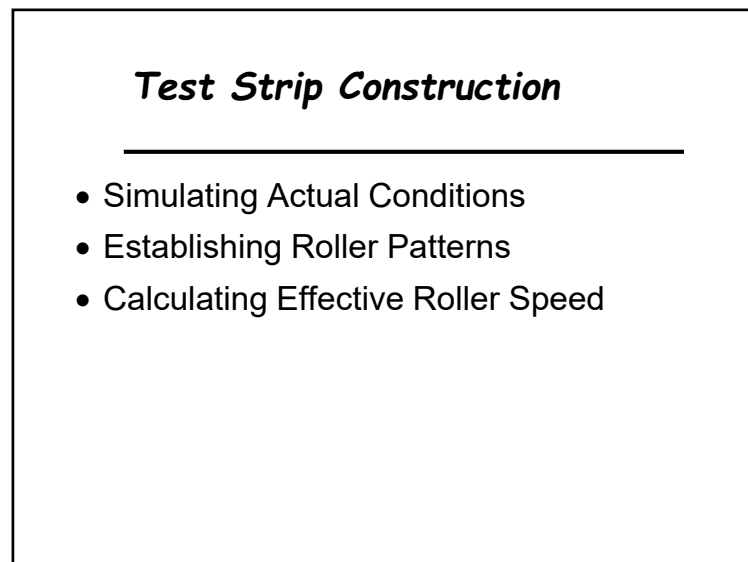
80



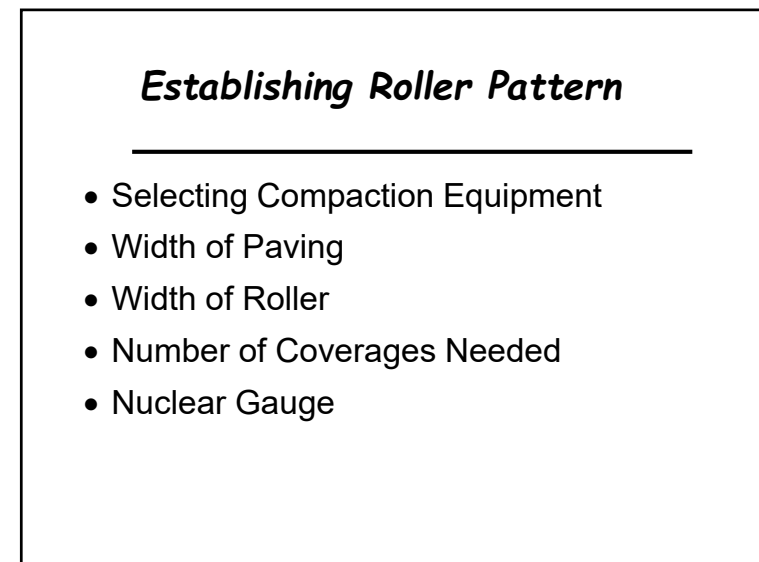
81



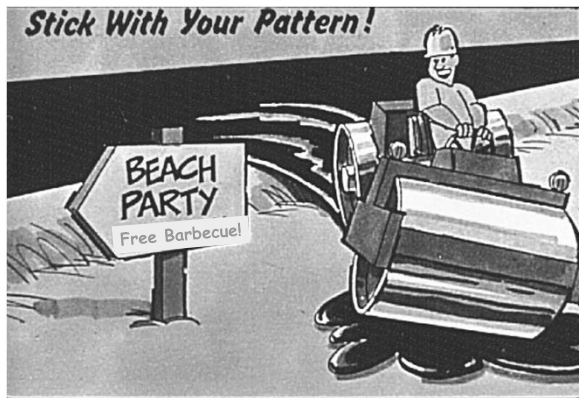
82



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Roller Types by Application

Breakdown Rolling

Static (tons)	Pneumatic (Wheel Size)	Vibratory (tons)
8.0 to 10.5	20 in rim	10 to 11
8.0 to 12.0	24 in rim	> 12.5
10 to 7		6 to 8

Pneumatic can be used for breakdown of tender mixes but the start temperature should be lower.

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Roller Types by Application

Intermediate Rolling

Static (tons)	Pneumatic (Wheel Size)	Vibratory (tons)
8 to 12	15 in rim	6 to 8
10 to 14	20 in rim	10 to 11
	24 in rim	> 12.5

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Roller Types by Application

Finish Rolling

Static (tons)	Vibratory (tons)
	Static Mode
8 to 10.5	6 to 8
8 to 12	10 to 11
10 to 14	> 12.5

88

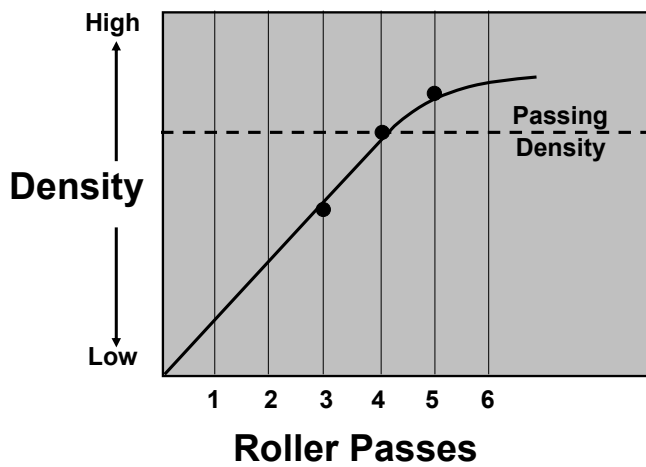
**How Many Repeat Passes
to Assure Density?**

89

Checking Density

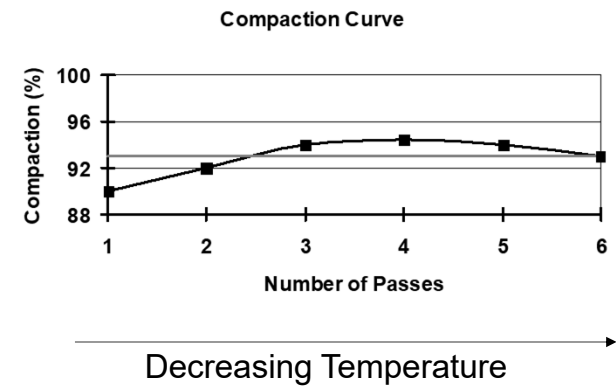


90

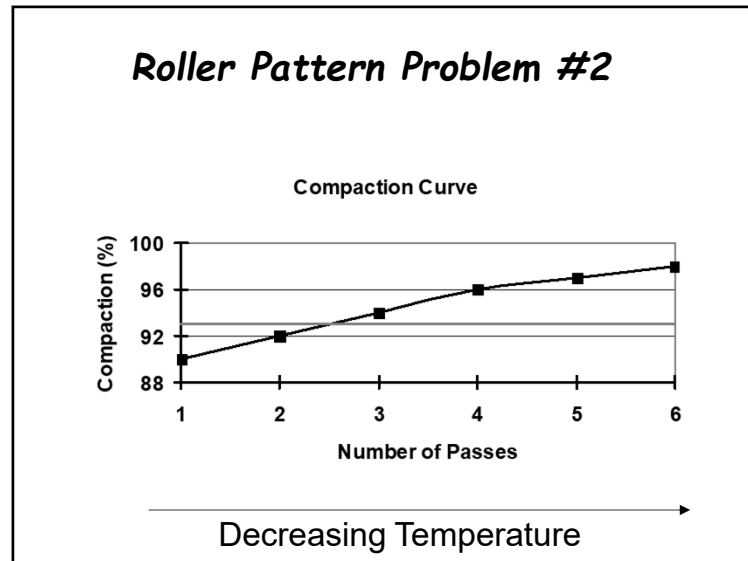


91

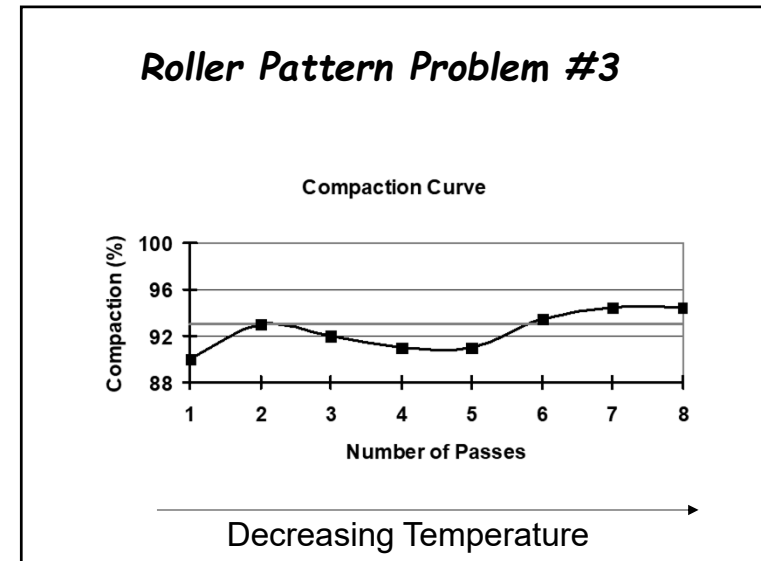
Roller Pattern Problem #1



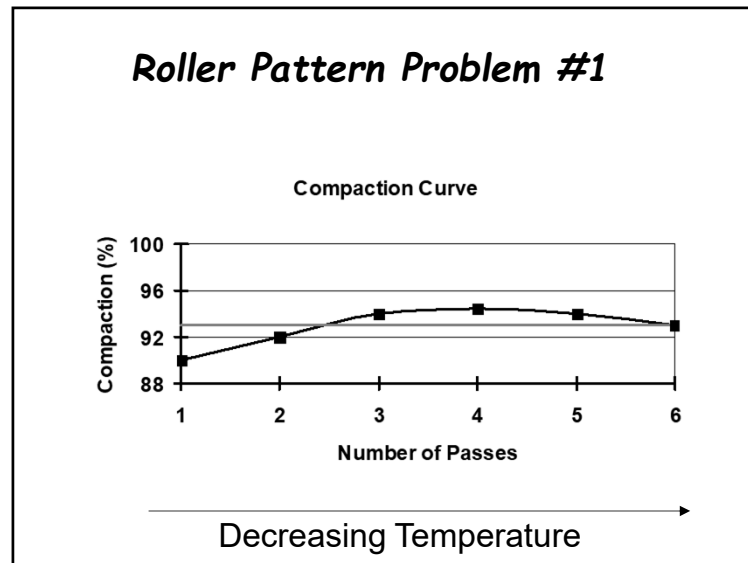
92



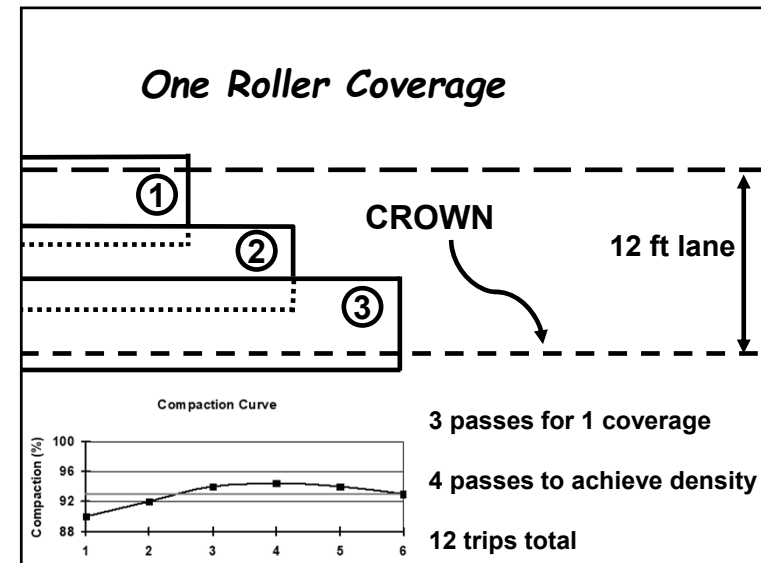
93



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95



96

Temperature Gauges



97

Interior Temperature



98

Improper Gauge Usage



99

Coring



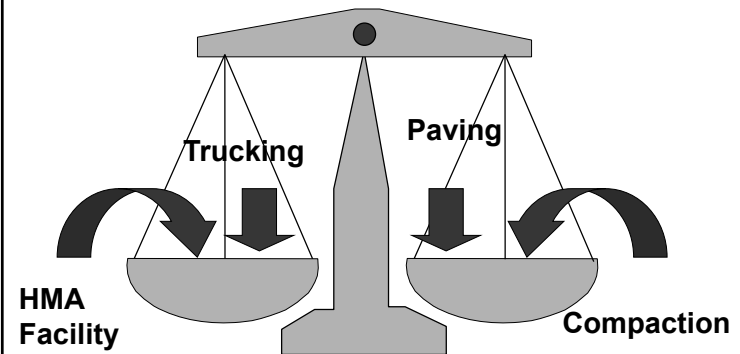
100

Topics

- Factors affecting compaction
- Time available for compaction
- Roller types
- Roller operations
- Roller pattern
- Production rate
- Roller maintenance

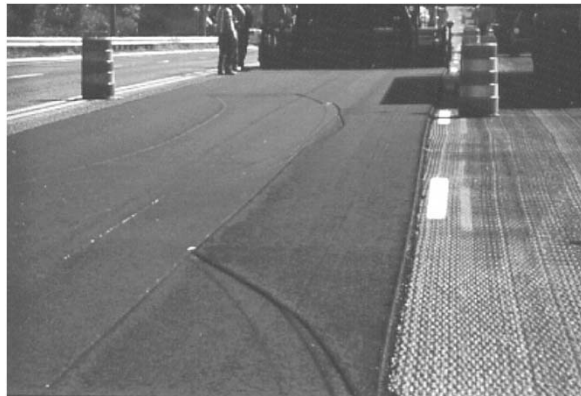
101

Balancing Production



102

Rolling Zone



103

Calculating Your Rolling Zone (Vibratory roller)

- Estimate roller speed using frequency and impacts per foot:
 - Frequency = 2800 vpm
 - 12 impacts per foot
 - Roller speed = $2800/12 = 233.3$ fpm (2.7 mph)
- Adjust for reversing factor
 - Roller speed * (1-reverse factor/100)
 - 13% reverse factor
- Effective roller speed = $233.3 * (1-13/100) = 203$ fpm

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Calculating Your Rolling Zone

- Effective roller speed = 203 fpm
- Effective Compaction Rate =
 - Effective roller speed/(number of passes per coverage)
 - Number of passes for coverage
 - 3 passes to cover
 - 3 coverages for density
 - 9 total passes
 - Adjust for roller efficiency
 - Roller efficiency = 63%
- Effective compaction rate = $203/9 \times (63/100) = 14.2$ fpm

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Calculating Your Rolling Zone

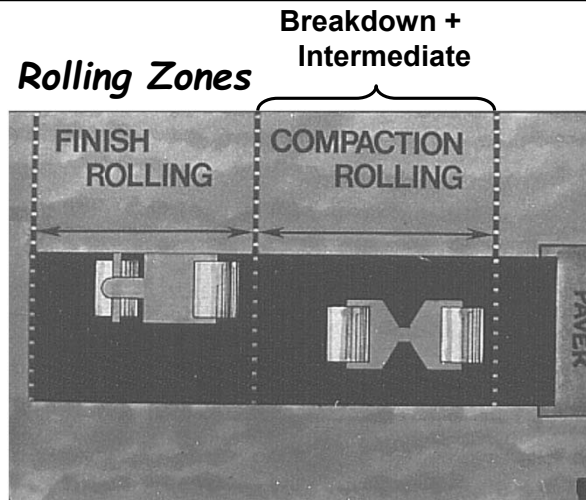
- Roller rate = 14.2 fpm
- Time available for compaction (TAC)
 - From Environmental Variables chart
 - Or PaveCool
 - Example: 10 minutes
 - 2 in thick mat
 - mix temperature of 250 °F
 - base temperature of 50 °F.
- Rolling zone
- Roller rate x TAC = 14.2 fpm X 10 minutes = 142 ft

106

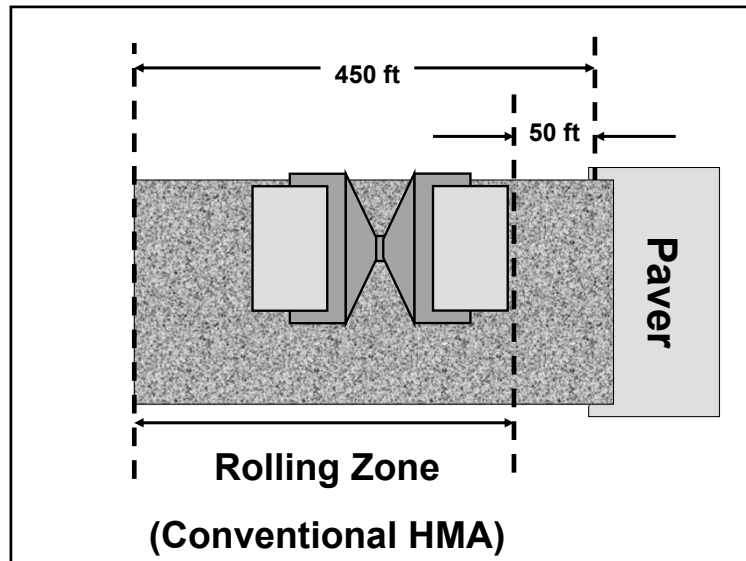
Balancing the paving example

- A roller rate of 14.2 fpm would control the productivity of the paving operation
- This is a low production speed and is probably not acceptable.
- What can be done???

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109

Roller Production Rate Problem

MIX DELIVERY RATE		PAVING RATE		ROLLING RATE	
Plant Rate Avail.	300 tph	Paving Width	12 feet	VPM (highest)	2800 vpm
Total Mix	3000 tons	Paving Thickness	3 inches	Impacts/foot (10-12)	10 impacts/ft
Total Paving Time	10 hours	Reference Density	150.5 pcf	Reverse Factor (100%)	100%
Mix Rate OK?					1.0 fpm
Truck Cycle					1.0 fpm
Total Trip					84 inches
Prep/Wait					6 inches
Load Time					50 feet
Ticket/Tag					2
Haul Time					3
Wait @ J					7
Dump/clean	5 min.				83 %
Return Haul	25 min.			Roller Rate	22.7 fpm
Truck Cycle	1.17 hours/trip			Rolling Zone	200
# of Loads	8 loads/truck			Time Elapsed	8.8
# of Trucks	25 trucks				
OK?	Exactly enough trucks				

Spreadsheet for balancing production

	Paver	Roller
Production Rate (fpm)	22.7	22.7
Production Rate (sq.yd/hr)	1817	1814

STOP!! - Whoo Pete!
Paver's outrunning Roller

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- ### Topics
- Factors affecting compaction
 - Time available for compaction
 - Roller types
 - Roller operations
 - Roller pattern
 - Production rate
 - Roller maintenance

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- ### Roller Maintenance
- Water Systems
 - Hydraulic Systems
 - Mechanical Systems
 - Vibratory Systems
 - Rolls, Tires, Pads, Scrapers

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Water Spray Bar



113

Pads



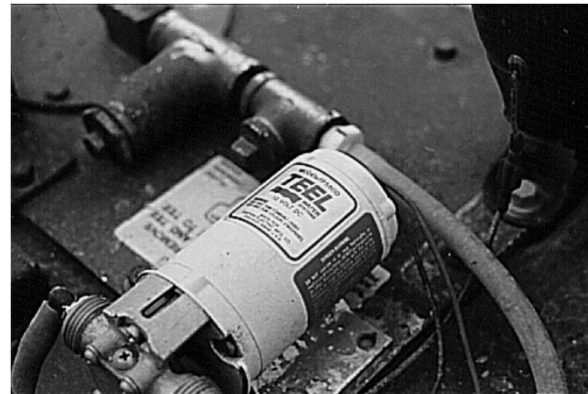
114

Poor Maintenance



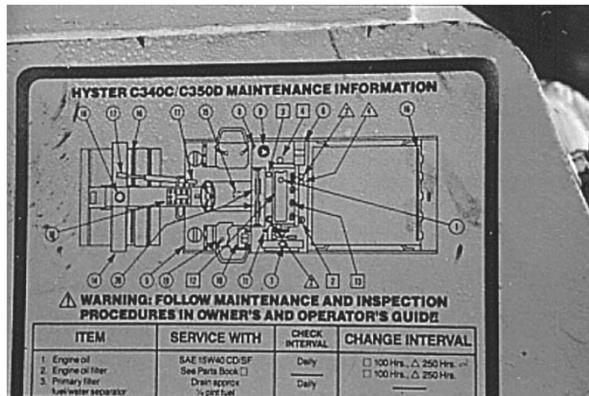
115

Water Pump



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Maintenance Chart



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Summary notes

- Maintaining proper speed
 - Roller speed has one of the greatest influences on mat quality.
 - too slow can over compact the surface
 - too fast can leave gaps and compromise safety if the operator gets too close to the paver

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FORM 8-1 ROLLER MAINTENANCE CHECKLIST Page 1 of 1

Roller ID#: _____ Date: _____

CHECK DAILY

☐ (1) **COOLING SYSTEM** - (a) Check coolant level; (b) Check radiator core for plugging with dirt or oil; (c) Check hoses for cracking and leaking; (d) Check belts for proper tension; (e) Check fan for loose bearings. Fill, adjust and/or replace as needed.

☐ (2) **ENGINE LUBE SYSTEM** - (a) Check engine oil level; (b) Check for any leakage

Roller maintenance checklist

☐ (4) **HYDRAULIC SYSTEM** - (a) Check oil level; (b) Check and look for any leaks around pumps, lines, fittings, and filters. DO NOT OVERTIGHTEN.

☐ (5) **ELECTRICAL SYSTEM** - (a) Look at the wiring and connections for being loose or oily. If oily, spray with contact cleaner. (b) Look at battery cable connections and clean if starting to corrode.

☐ (6) **ROLLS & TIRES** - (a) Check rolls for uneven wear; (b) Check tires for uneven wear and maintain at ____ kPa air pressure (cold/hot); (c) Check scrapers and mats (replace as necessary).

☐ (7) **WATER SYSTEM** - (a) Check fill areas for debris; (b) Check filters for contamination and clean; (c) Test system to see if all nozzles are clear and operational.

☐ (8) **INSPECTION** - (a) Look for loose bolts and tighten right away when found; (b) Clean out scraper and mat areas after each day's run; (c) Keep top deck and operator's station clear; (d) Check for other oil or grease leaks from gear boxes, transmission, differential, axle hubs, and report to shop immediately.

CHECK WEEKLY

☐ (1) **BRAKE SYSTEM** - (a) Check master cylinders for hydraulic brake fluid level. Keep filled to proper level.

☐ (2) **ELECTRICAL SYSTEM** - Batteries: (a) Check water level and maintain level above the plates. If one or more cells keeps going dry, it means the battery is near the end of its life.

☐ (3) **TRANSMISSION** - Gear boxes and pump drive housing: (a) Check grease levels and fill as needed to proper level.

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Summary notes cont'd

- Maintaining proper speed
 - Roller speed has one of the greatest influences on mat quality.
 - Using the correct speed will also help to maintain the appropriate impact spacing to prevent washboarding. For most jobs, correct spacing is between 10 and 12 impacts per foot.
 - Even if the washboarding isn't readily seen at the time the road is paved, over time, it increases its affect because of the way the asphalt continues to compact

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Summary notes cont'd

- Temperature
 - Cannot improve the density of cold mats!!!
 - Breakdown >240F
 - Intermediate 240 – 190
 - Finish 190 – 175 (165)

121

Summary notes cont'd

- Good vibrations
- Vibratory compaction creates shock waves that compact from the bottom up
- Static compaction works from the top down.
- Most jobs use a combination of compaction modes,
 - start with vibratory compaction
 - finishing with static
- Select the right amplitude
 - amplitude settings are determined by the depth of the lift
 - Lower amplitude settings are recommended for lifts of 2 in. or less;
 - higher amplitude settings should be used for deeper lifts.

122

Summary notes cont'd

- Follow the rolling pattern
 - Gradual turns
 - Gradual acceleration and breaking
- Always stop at an angle to the direction of paving

123

Finally

“The single most important thing you can do to a pavement is compact it.”

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Opening to Traffic



125

INSPECTOR'S DUTIES - ASPHALT PAVING

*Modified by WVDOH,
APAWV, and WVU ATP*

Work Includes...

■ **Resurfacing Projects**

- State Funded
- Federal Funded
- Purchase Order
Paving
- Bond Projects
(Roadway
Reconstruction)

■ **New Projects**

- New Roads
- Bridge Approaches
- Road Widening
- Intersection
Improvements
- Bond Projects
(Construction)

Funding Programs

■ **Federal Aid Funding (National Highway System)**

- Interstate
- APD
- Federal Aid Other
 - US routes
 - State NHS routes

■ **State Funding**

- **SLS** – State and Local
service Routes
(County Routes)
- Non-NHS State
Routes

**Inspector duties vary depending on the
funding program, route, compaction method**

Basic HMA Applications

- **Base Course** – stone or HMA course placed above the subgrade or subbase of a road and helps to further distribute the load and improve the overall structure of the pavement section.
 - Marshall Base 1 and 2
 - Superpave Base 37.5, 25 and 19
- **Wearing Course** – a single lift of constant thickness that is to be placed over the entire pavement surface and serves as the riding surface, receives the highest concentration of stress.
 - Marshall Wearing 1, 3, and 4
 - Superpave 4.75, 9 and 12.5

Basic HMA Applications

- **Patch & Level (P&L)** – placed at various locations throughout the project to remove irregularities in the existing pavement, such as dips, or to raise the outside edge of pavement to improve the template prior to placing a base or wearing course.
- **Scratch Course** – a leveling course used for deviations less than an inch and can be placed over the entire length of the project.



Edge Failure Repairs



1" thin overlay



1.5" Overlay with paved shoulders

Activities Prior to and during Construction...

- Bond and Insurance Checked – Issue NTP
- All Requested Pertinent Paperwork
 - Key Personnel
 - EEO
 - DBE Plan for Participation
 - Waste and/or Borrow Pit Agreements (SHPO)
 - Pollution and Erosion Control Plan
 - QC Plan - Available? Approved?

Resurfacing Inspector may be requested to verify these items on the project.

Activities Prior to Construction...

- Construction Layout
 - Staking the project
 - Mark Heel-ins
 - On larger projects this is usually included in contract
- Maintenance Finished?
 - Have all pipes been placed, ditches pulled, mowed, etc
- Verify Quantities In plans with Actual Field Measurements.

Activities Prior to Construction...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



Activities Throughout the Project...



HMA Inspector's Requirements

- Generally needs to be trained in all aspects of materials and construction
 - Hot-mix Asphalt
 - PCC
 - Soils and Compaction
 - Environmental
 - Traffic Control
- **However, it is not a good idea to serve as the inspector and the Compaction Tech, etc.**

HMA Inspector's Requirements

- Must have an understanding of the entire paving operation
 - Plant Operations
 - Hauling Limitations
 - Bridge Postings
 - Truck weights
 - Paving Equipment
 - Paver (Screed)
 - Rollers

HMA Inspector's Requirements

- HMA Inspector has the same general duties regarding execution of the contract, but they do not have a field office. All forms, typicals, specs, etc. are kept in the “mobile field office.”
- Good Inspector can visualize the entire job from beginning to end
 - Foresee contract issues ahead of time so that a plan of attack can be implemented right away

HMA Inspector's Requirements

- Materials and the JMF – Job Mix Formula (Approved Mix Design)
 - Temperature Range established for the mix
 - Maintain communication with the plant inspector regarding other properties as well
 - Max Density
 - Lab Number
 - All mix designs are “Verified” at the beginning of each season

Good Workmanship



Proper Edge



What you don't want to see



What you don't want to see



Night Work

- More work is being done during night hours than in the past
 - Especially in congested areas on major routes
 - Everything is more difficult to inspect at night
- Challenge the inspector faces is much greater
- Cooler temperatures can be a killer on compaction

Night Work



Contractor's Requirements

- Contractor should have an overall paving plan including...
 - Production Rate
 - Haul Distance
 - Number of Trucks
- Properly staffed paving crew
 - Paving operator
 - Screed person
 - Broom
 - Compaction
 - Laborers

Contractor's Requirements

■ On-site QC technician

- Fully understands the process of density compliance and the operation of a nuclear gauge
 - Do they know the difference between “Lot-by-lot” and “Rollerpass” testing?
 - Are they capable of implementing the thin lift correction factors properly?
- *Field Sampling Requirements for PWL*
- Good technician understands the limitations of the JMF and has a general understanding of the nature of the specific mix

Activities Prior to Construction (the day of paving)...

- Traffic Control Check – need to maintain a good flow of traffic but maintain a *SAFE WORKZONE*.
 - Check all arrow boards and message boards, Type B lights
 - Make sure flaggers understand the scope of traffic
 - Remember *Night-time* Limitations
- Check Heel-ins
- Check surface conditions
- Tack Distributor – good condition
- Other Equipment?

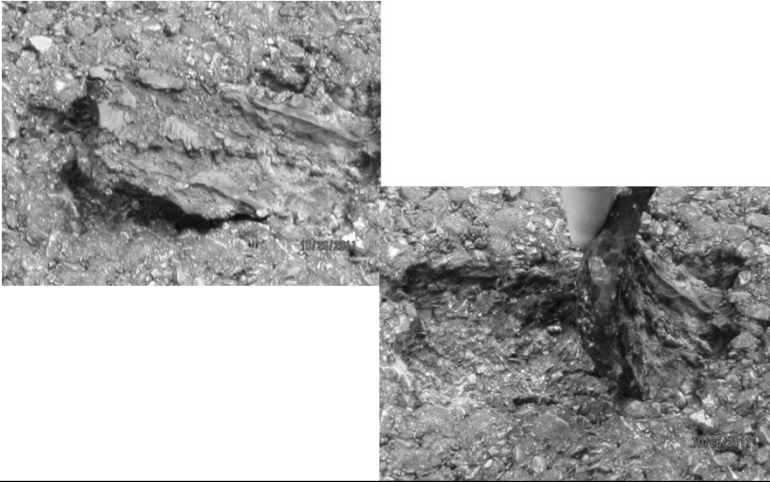
Activities Prior To and During Construction

- Check traffic control
- Check ground and air temperatures
- Check delivery tickets for correct Mix Design, Materials, and Project Number
- Check HMA temperature in the truck
- Check the depth of HMA as being placed

Inspector's Daily Duties...

- Collect tickets – JMF# and target density, truck weights
- Verify Rollerpass or Lot-by-lot
- Observe mix characteristics – mat texture, segregation, flushing, contamination (*Night-time!*)
- Mat - screed setting and mat thickness
- Paver must maintain a constant head of flow
 - Best to stop and start quickly

Contamination!!!



Contamination!!!



Issues Caught During Construction Can Be Corrected



Inspector Daily Duties...

- Be on the project from before work begins to after everything is off the road for the day.
- Get with the contractor's foreman at the end of the day to agree on quantities and hours.
- Document any changes made on project in your DWR whether big or small and note if someone authorized the changes.

Inspector Daily Duties...

- Throughout day check flagger placement and pilot truck return times to keep traffic flowing as smooth as possible.
- Document any issues that come up during the day on your DWR. Issues might not seem important at the time but could be something major down the road.

Inspector's Daily Duties...

- Is compaction being done properly and are density readings acceptable
- Application rates at minimum 2500' intervals
- Placement of temporary tape and temporary markings as needed/required
- Throughout day check your Traffic Control to make sure signs are kept up in proper work zone specifications

Inspector Daily Duties

- Most flagging subcontractors have sheets to sign daily for their time, if not make sure you talk to the one in charge of the crew to let them know their hours for the day.
- Keep a notebook for yourself of quantities placed daily to easily reference back to on project.
- Get DWR submitted daily!

Inspectors Office Duties

- Check payrolls for contracts to verify correct pay.
- Initiate change orders for projects
- Make sure all samples taken and approved sources are entered in Site Manager
- Correct mistakes on DWR in a timely manner so estimates can be run.

HMA Approximate Rate and Lift Thickness

- 1.0" Compacted = 110 PSY
- 1.5" Compacted = 165 PSY
- 2.0" Compacted = 220 PSY

Verifying Quantities

- Given: A Two Lane Road is 1 mile long, has an average width of 28', 1.5" Overlay. How many Tons per lane of HMA?

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- Given: A Two Lane Road is 1 mile long, has an average width of 28', 1.5" Overlay. How many Tons per lane of HMA?
 - Convert miles to Lineal Ft (1 mile x 5280 ft/mi)
= 5,280 FT (End Station would be 52+80)

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= 1,355.2 TN

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 - $\frac{16,426.67 \text{ SY} \times 165 \text{ PSY}}{2000 \text{ LB/TN}}$
= 1,355.2 TN
- ANSWER = $1,355.2 \text{ TN} / 2 \text{ lanes} = \underline{677.6 \text{ TN per lane}}$

After Project Completion

- Verify all payrolls are received and checked for compliance.
- Get final paperwork done in a timely manner.
- Get Over/under change order ready to be processed.
- Make sure any outstanding change orders are paid when they complete the process.

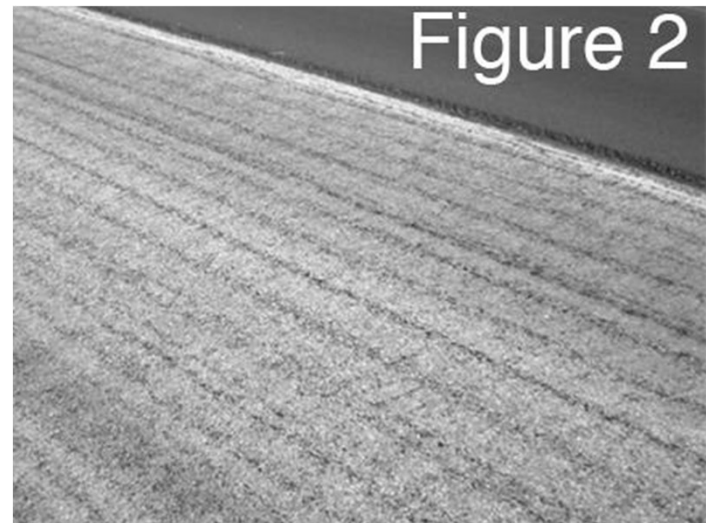
What is the Proper Rate for Tack?

What is the Proper Rate for Tack?

- Tack Coat Specification
 - Gives guidance regarding “break” and “set”
 - Discusses rate of dilution
 - Shows a table with application rates based on paving surface
- We want to achieve a *desirable* “residual” asphalt content
- How do you calculate tack application rate?



Good Tack Application



Poor tack application



Is this a good coat?

TABLE 408.11

Condition of Existing Pavement	Application Rate (gal/sy) ^(Note 2)	
	Undiluted	Diluted (1:1) ^(Note 3)
New HMA ^(Note 4)	0.04 – 0.05	0.08 – 0.10
Oxidized HMA	0.07 – 0.10	0.13 – 0.20
Milled Surface	0.10 – 0.13	0.20 – 0.27
PC Concrete	0.07 – 0.10	0.13 – 0.20

Note 2: Application rates are for slow setting emulsions grades (SS and CSS) that contain approximately 60% asphalt material. Rapid setting emulsion grades may contain slightly higher or lower asphalt contents, but can usually be applied within the same application range.

Note 3: Dilution rate only applies to SS and CSS grades.

Note 4: Tack coat is normally not needed over a layer of new HMA that has been placed within the last few days, as long as the underlying new layer has not become dirty under traffic or from windblown dust.

Example Tack Calculation

- Existing roadway is very old, dried, cracked, etc. – “Oxidized” from Table 408.11
- Prior to tack placement, dial gauge reads 450 gallons. Tack is diluted.
- Tack is placed from Sta. 0+00 to Sta. 10+00. Dial gauge reads 275 gallons after tack placement.
- Road width is an average of 12’.

Example Tack Calculation

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408.12-METHOD OF MEASUREMENT:

No materials shall be removed from the Project for any purpose until the operation has been completed and the quantities of materials incorporated into the operations have been determined, except when authorized by the Engineer.

The quantity of “Asphalt Material” for tack coat shall be the number of gallons, prior to dilution, incorporated into the completed work. Any applicable dilution rates, shall be supplied to the Engineer by the Contractor on the material delivery ticket.

Dry sand or stone chips used as a blotter course due to excessive use of tack coat shall be considered incidental to the work.

When items for maintaining traffic are included in the Contract, they will be measured and paid as provided in Section 636.

Example Tack Calculation

- Calculate the tack used

$$\text{PAY QTY.} = 450 \text{ gal} - 275 \text{ gal} = \underline{175 \text{ GALLONS}}$$

- LETS CHECK THE APPLICATION RATE:

$$- (12' \times 1000') \div 9 \text{ ft}^2 \text{ per yd}^2 = 1333.33 \text{ yd}^2$$

- Calculate Rate of Application

$$- 175 \text{ gal} \div 1333.33 \text{ yd}^2 = \underline{0.13 \text{ gal/yd}^2}$$

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Contract: _____ Inspector: _____ DWR Date: 06/17/19
 Project Nbr: _____ Line Item Nbr: 0015 Loc Seq No: 1
 Item Code & Desc: 408002-001 ASPHALT MATERIAL

West Virginia Department of Transportation
 Division of Highways
 Inspector's Bituminous Emulsion Tack Worksheet
 Producer / Supplier: MPTC.01.705 - Marathon Petroleum F-1 @ Cabellsburg Total Quantity Placed: 175.

Ver 1.1 March 2 2016

Row 1 of 1	Ticket Number	Original Invoice No	Material Type	Source of Material
OBSERVATIONS -	Comment below if any of the following are not met:			
Traffic Control and Flaggers in place	☒			
Surface temp above 40 degrees F	☒			
Surface clean prior to placement	☒			
Uniform application of tack coat	☒			

Existing Pavement Condition	Target Application Rate (gal/yd ²)	
	Undiluted ☒	Diluted (1:1) ☐
New HMA ☐	0.04 - 0.05	0.08 - 0.10
Oxidized HMA ☒	0.07 - 0.10	0.13 - 0.20
Milled Surface ☐	0.10 - 0.13	0.20 - 0.27
PCC ☐	0.07 - 0.10	0.13 - 0.20

*Undiluted = 60% Residual Asphalt, Diluted = 90% Residual Asphalt

A	B	C	D	E	F	G	H	I	J
Time	Start Station	End Station	Length (ft)	Width (ft)	Area (yd ²)	Initial Reading (gal)	Final Reading (gal)	Amount Applied (gal)	Rate (gal/yd ²)
00:00	0 +00	10 +00	1000	12	1333.33	450	275	175.00	0.13

Remarks: _____ Running Amount Applied: 175.

EXAMPLE DWR ENTRY

01/02/2024 11:00:19

Division of Highways

Daily Work Report Report v1

Contract: 2023080002, BIG OTTER-FRAMETOWN RD Prime Contractor: WEST VIRGINIA PAVING, INC.
 DWR Date: 9/6/2023 Day of Week: Wednesday Inspector: James Putnam Agency Project Engineer: Steven Ruppert
 Seq Num: 1 DWR Status: Approved Entered By: James Putnam Agency Delivery Engineer: Steven Ruppert
 Last Updated Date: 9/14/2023 Last Updated By: Savannah Summers Managing Office:
 Weather: 9 Low Temp: 66 High Temp: 91 Rainfall Amount: SECIR Indicator:
 Fed. Proj. Num: NHPP-0079(091) State Proj. Num: S306-79-43.37 00
 Work Items: Yes Contractors: Yes Daily Staff: No Attachment(s): Yes

Remarks

Type	Remarks
00	PRIME CONTRACTOR TIME WY PAVING NO LUNCH 6:00AM TO 7:00PM = 13 HRS
	WDOH PERSONNEL TIME JAMES PUTNAM 5:30AM TO 7:30PM = 13.5 HRS 1 HR TRAVEL 12.5 HRS PROJECT INSPECTION, DOCUMENTATION, ETC
	WDOH PERSONNEL TIME DAKOTA RUFFLE 6:30AM TO 7:30PM = 12.5 HRS 1 HR TRAVEL 11.5 HRS PROJECT INSPECTION, ETC
02	2023 09 06 DWR 0025 SUPERPAVE ASPHALT TICKETS TY19
03	2023 09 06 DWR 0025 SUPERPAVE ASPHALT DELIVERY REPORT TY19 LINE # 0025, ITEM 410001-030 SUPERPAVE ASPHALT BASE TY 19

Contract: 2023080002 DWR Date: 9/6/2023 Seq. No.: 1 Page: 1 of 8



Division of Highways

01/02/2024 11:00:19

Daily Work Report

Report v1

Item Postings

Item Description	Item ID	Seq. Num	Cont. Ln	Project	Category	Proj. Ln	Quantity Posted	Unit	From Station - To Station Location
SUPERPAVE ASPHALT BASE CRSE, TY 19	410001-030	1	0025	2023080002	0001	0025		SY	BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location
	19 mm Asphalt Mix, Superpave		TN		

Contractor: WEST VIRGINIA PAVING, INC.

Comments: PAY FOR THIS ITEM WILL BE MADE AT A LATER DATE ON COMPLETION OF PWL SAMPLE TESTING.

AT THE FOLLOWING LOCATIONS, THE CONTRACTOR USES A TRANSFER MACHINE AND A RUBBER TIRE PAVEMENT PAVING MACHINE TO PLACE ASPHALT BASE FOLLOWED BY 3 STEEL DRUM ROLLERS FOR COMPACTION. TERRADON TECHNICIAN, JUSTIN WHITTINGTON, ON SITE FOR DENSITY. WV PAVING, JAMEY PROPPS ON SITE FOR PWL CORE DRILLING. WYDOH TECHNICIAN DAVID SCHOOLCRAFT ON SITE.

(PAYMENT WILL BE REQUESTED FOR A 12FT LANE)

SB SLOW LANE

2611+75 TO 2596+70 = 1505

EQUALITY

2596+75 TO 2516+35 = 7943

1505 + 7943 = 9448

(9,448 X 12 = 113,376) / 9 = 12,597.333

NB SLOW LANE

2516+35 TO 2522+80 = 645

(645 X 12 = 7,740) / 9 = 860

12,597.333 + 860 = 13,457.333 SY

Contract : 2023080002 DWR Date: 9/6/2023 , Seq. No.: 1 Page: 4 of 8



Division of Highways

01/02/2024 11:00:19

Daily Work Report

Report v1

PAY QUANTITY = 0 SY
TRAFFIC CONTROL DEVICE 636011-001 1 0140 2023080002 0001 0140 0 UN BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location
	Traffic Control Devices	0	EA		

Contractor: HIGHWAY SAFETY, INC.

Comments: ALL TRAFFIC CONTROL DEVICES ARE PROPERLY LOCATED THROUGHOUT THE PROJECT. THE SIGNS ARE CLEAN, LEGIBLE, AND CLEARLY VISIBLE. SPEED TRAILERS AND ARROW BOARDS ARE ALSO UTILIZED IN EACH LANE AS PART OF THE REQUIRED SPECIFICATIONS FOR THE TRAFFIC CONTROL SET UP AS LISTED PER THE PLAN SHEET. REFER TO THE DAILY CHECK LIST FOR ADDITIONAL INFORMATION.

PAY QUANTITY = 0 UN
TRAFFIC DIRECTOR 636014-002 1 0155 2023080002 0001 0155 11.5 HR BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location

Contractor: WEST VIRGINIA PAVING, INC.

Comments: CONTRACTOR USES A TRAFFIC DIRECTOR ON THIS DATE FOR THE SAFETY OF THE TRAVELING PUBLIC. PATROL VEHICLE FOLLOWED THE OPERATIONS THROUGHOUT THE PROJECT AS PER THE CONTRACTOR'S REQUEST. THE OFFICER ON DUTY THIS DATE IS BRIAN YOUNG.

HOURS ON SITE 7:00AM TO 6:30PM = 11.5 HRS

PAY QUANTITY = 11.5 HR
ELECTRIC ARROW 636021-001 1 0160 2023080002 0001 0160 2 DA BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location

Contract : 2023080002 DWR Date: 9/6/2023 , Seq. No.: 1 Page: 5 of 8



Division of Highways

01/02/2024 11:00:19

Daily Work Report

Report v1

Comments: CONTRACTOR UTILIZES 32 TYPE B WARNING LIGHTS. ALL TYPE B WARNING LIGHTS ARE PLACED PROPERLY. ALL TYPE B LIGHTS ARE ON AND WORKING PROPERLY AS PER THE PLANS. B LIGHT PAYMENT WILL BE REQUESTED FOR PLACEMENT ON ROAD WORK 2 MILES SIGNS, ROAD WORK 1 MILE SIGNS, AND 55 MPH SIGNS.

ALL B-LIGHTS ARE WORKING THIS DATE

PAY QUANTITY = 32 DA

SHADOW VEHICLE 636028-002 1 0175 2023080002 0001 0175 MO BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location

Contractor: WEST VIRGINIA PAVING, INC.

Comments: CONTRACTOR UTILIZES A SHADOW VEHICLE WHILE PERFORMING WORK. I PREVIOUSLY ADVISED WV PAVING TO HAVE THE SHADOW VEHICLE SET UP TO MEET SPECIFICATIONS LISTED IN THE STANDARD DETAILS. THE SHADOW VEHICLE HAS BEEN EQUIPPED WITH FLASHING BEACONS AND A TRUCK MOUNTED ATTENUATOR. SPOKE WITH VERN WALKER, WV PAVING, ABOUT HAVING 2 SHADOW VEHICLES ON THE PROJECT WHEN PROJECT SCHEDULED FOR 1 AS PER THE PLANS. WV PAVING AGREED HAVING PAYMENT FOR THE USE OF 1 UNLESS 2 BECOMES NEEDED. ONLY 1 IS BEING UTILIZED AT THIS TIME.

PUT IN SERVICE ON 07/25/2023. NEXT PAYMENT REQUEST 9/25/2023

PAY QUANTITY = 0 MO

SPEED MONITORING TRAILER 636030-001 1 0180 2023080002 0001 0180 2 DA BIG OTTER FRAMETOWN RD PROJECT

Materials:	Material Name	Installed Quantity	Unit	Source	Work Location
	Traffic Control Devices	0	EA		

Contractor: HIGHWAY SAFETY, INC.

Comments: THERE ARE 2 SPEED MONITORING TRAILERS PLACED ON THIS PROJECT. ONE PLACED ON THE SB SIDE OF THE PROJECT AND ONE ON THE NB SIDE OF

Contract : 2023080002 DWR Date: 9/6/2023 , Seq. No.: 1 Page: 7 of 8

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
INSPECTOR'S ASPHALT PAVEMENT WORKSHEET

AUTH NO: _____ PROJECT NO: _____ IRI NO: _____
LINE NO: _____ ITEM NO: _____ DATE: _____

LOAD	TICKET #	TONS / BG	CUM. TOTAL	LOAD	TICKET #	TONS / BG	CUM. TOTAL
1			25				
2			26				
3			27				
4			28				
5			29				
6			30				
7			31				
8			32				
9			33				
10			34				
11			35				
12			36				
13			37				
14			38				
15			39				
16			40				
17			41				
18			42				
19			43				
20			44				
21			45				
22			46				
23			47				
24			48				

REMARKS: _____

TOTAL Tons (kg) THIS DATE: _____ INSPECTOR: _____
Signature

ROLLER PASS DATA	PCF	SPEED OF ROLLER	MPH TYPE OF ROLLER
ROLLER PASS DATA	PCF	SPEED OF ROLLER	MPH TYPE OF ROLLER
			PLAN THICKNESS

CALCULATION OF APPLICATION RATE (ONE CALCULATION PER EACH 100 ft (300 LF))

SECTION STATION					
END STATION					
INCHES					
FEET					
SECTION					

OBSERVED SIMILARITY TESTS (ONE OBSERVATION PER EACH 100 ft (300 LF))

LOG NUMBER					
TEST NUMBER					
SECTION					

MAT THICKNESS (PRIOR TO COMPACTION) & MAT TEMPERATURE (AT TIME OF FINAL COMPACTION PASS) CHECKS (ONE CHECK PER 300 ft (300 LF))

TIME					
MAT TEMPERATURE					
MAT THICKNESS					
SECTION					

TIME					
MAT TEMPERATURE					
MAT THICKNESS					
SECTION					

ROLLER SPEED CHECKS (FOUR CHECKS, TWO AM AND TWO PM)

TIME					
ROLLER SPEED					

CHECKS ☐ VERIFIED PRIOR TO PLACEMENT

☐ RECORDS OF OPERATION REVIEWED

☐ TRAFFIC CONTROL DEVICES AND PLACEMENT IN PLACE

☐ ROLLER ADJUSTED AND PHOTOLOGS CORRECTED

☐ SURFACE CLEAN AND STRONGLINE PLACED

☐ ROLLERS AND FINESSE VERIFIED FOR COMPLIANCE

☐ CONTRACTORS IDENTIFY PROBLEMS ON SITE

☐ CONTRACTOR ADJUSTS PROBLEMS AND CORRECTS

CHECKS ☐ VERIFIED DURING PLACEMENT

☐ ROLLER COARED AND REGULATED WITH NO OIL LEAKS OR DAMAGED BARS

☐ OPERATING CONTROLS AND PAGES SPEED COMPARE TO PLANT PRODUCTION

☐ OPERATING SPEED ON-HIGH TOP OF MAT CORRECT

☐ CORRECT ROLLING RESOURCES BEING USED

☐ STRONGLINE ROLLER CHECKS BEING MADE

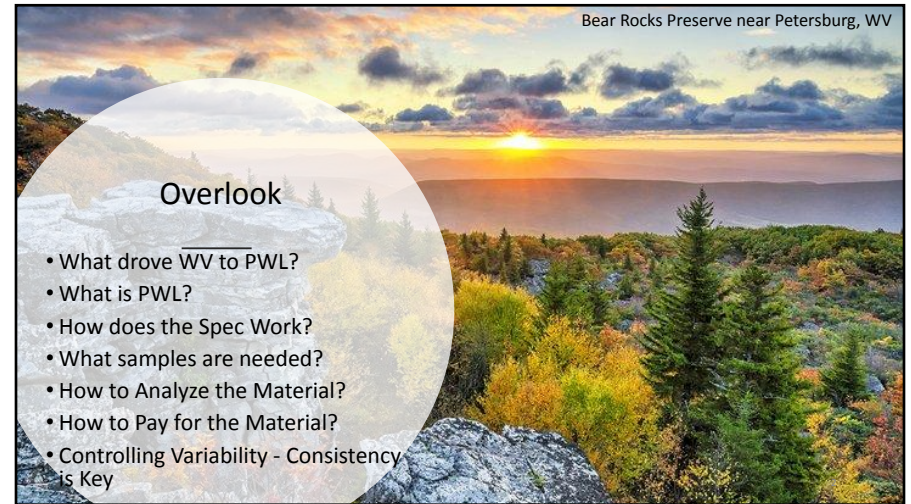
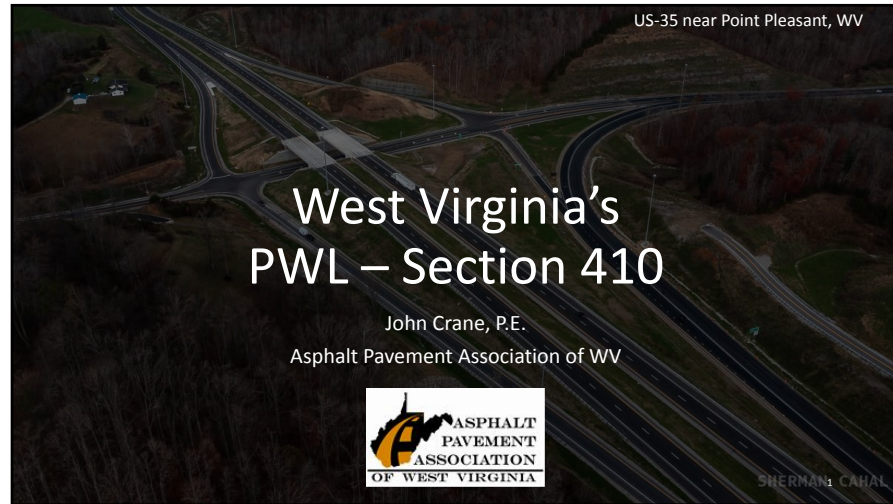
☐ COME BACK WITH QUALITY CONTROL PLAN MAINTAINED

☐ TEMPERATURE OF MATERIAL RECORDED ON TICKETS ONCE PER HOUR MINIMUM

AIR TEMP	°C (°F)	AT (TIME)		BASE TEMP	°C (°F)	AT TIME	
AIR TEMP	°C (°F)	AT (TIME)		BASE TEMP	°C (°F)	AT TIME	

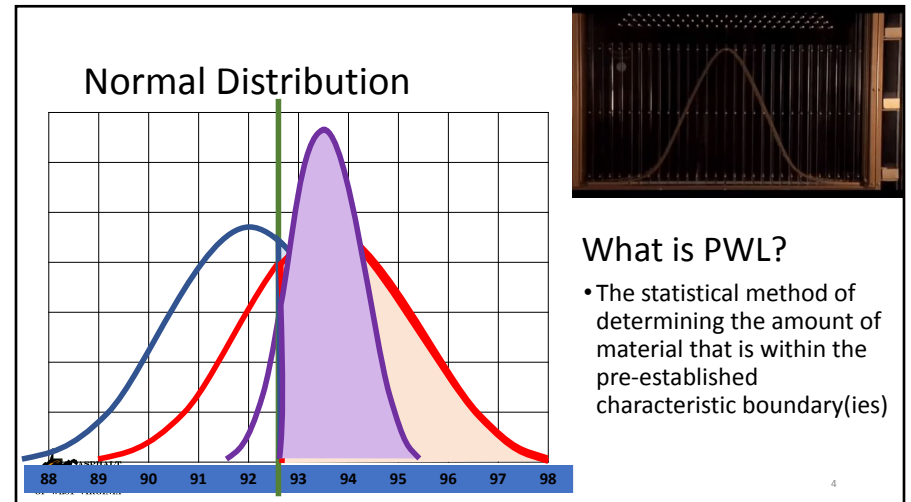


At the end of the day we want the same result.




Why West Virginia Changed their specs?

- Poor performing pavements with ineffective specification to combat issues
- FHWA's ~2011 Stewardship Review
 - Improve QA/QC procedures
 - Higher focus on DOH performing Acceptance testing of Materials
- DOH was investigating ways of achieve higher performing, longer lasting roadways
 - F&T testing
 - Standard Deviation Driven
 - Percent within Limitation (PWL)



Lets have some fun with statistics!



5

Why did WV go with PWL?

- Desired a more robust methods to Evaluate Overall Quality
- WVDOH's standard Asphalt QC/QA was based on moving average
 - Straight Average and Moving Average is not the best measure of quality
 - Lacks enforcement to stay on Target
- WVDOH wanted to focus on end results Specification drawing data from the field
 - Materials properties along with Mat and Joint Density
- Promote consistency throughout the project



PWL Paving – WV Overview

- This is the Method of QA for NHS routes and above
- Material samples are taken from the roadway
 - Asphalt Content / Gradation / Mat Density / Joint Density / Bond Strength / Thickness
- PWL statistical analysis
 - Based on average, standard deviation, and specified limits
 - Theoretical percentage of all data that is statistically within the overall specification limits
 - Not just an average or a moving average
- Pay Factors are established
- Payment is by the SY, not by the Ton
- Eliminates the need for change orders and over runs



7

What's **required** and What's not Quality Control (QC)

- Plant
 - Volumetric Mixture Control – Air Void, VMA, Asphalt Content, Gradation
 - Per tonnage basis – 1000-ton subplot (750 tons during mixture verification)
 - Samples must satisfy single sample tolerance and moving average tolerance
- Field
 - None*



8

What's required and What's not? Quality Assurance (QA)

- The WVDOH does all the Acceptance testing
- Field Sampled – 2500-ton Lots w/ 5 sublots
 - Thickness – Cores
 - Density Mat – Cores
 - Bond Strength – Cores
 - Asphalt Content – Loose Mix
 - Gradation (Dust Content) – Loose Mix
- Density Joint – Cores 10,000 FT Lot w/ 5 sublots
- Inspection @ Plant
 - Minimal involvement

Test	Limits
Asphalt Content	JMF $\pm$ 0.4%
Gradation (-%200)	JMF $\pm$ 2.0%
Density	91.5 – 97.0% G _{mm}
Joint Density	> 89.0% G _{mm}
Bond Strength	> 100 psi
Thickness	Design – 0.04"



9



PWL Paving – Loose Mix Samples

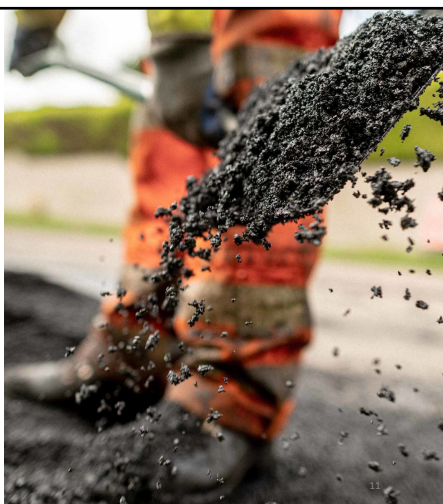
- Sampling Location
 - “Loose Mix” sample behind the paver for Pay
 - Still truck samples for plant QC/QA
- <https://www.youtube.com/watch?v=ITZeDmYojUuM>
- <https://www.youtube.com/watch?v=to8C-iesXkY>



10

PWL – Asphalt & Dust Content

- All materials arriving on the project are expected to be **quality** and stay **consistent**.
- Importance of Asphalt and Dust
- Asphalt and Gradations tests are performed in the lab
- Statistical Evaluation



PWL Paving – Core Samples

- Must use a 6-inch Core bit – Inside Diameter
- Cores are used to determine
 - In-place Density
 - Bond Strength
 - In-place Thickness
- Care should be taken to ensure the viability of the core
 - Drill as deep as reasonable possible
 - Do not pry or push on the layer to be tested.



12

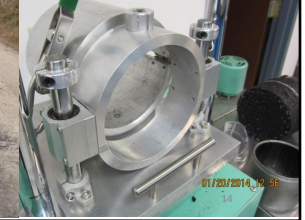
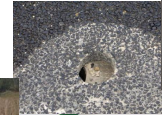
PWL Paving – Density Analysis

- Mat and Longitudinal Joint Evaluations
- Coring the finished pavement
- No field density testing is required
 - Still a good Practice for QC
 - Can use non-nuclear
- Density tests in the lab
- Statistical Evaluation



PWL Paving – Bond Strength

- Poor tack coats and surface prep can lead to premature failures
 - Layer slip / Delamination / Cracking
- Cores are tested in a Shear device
 - Lower limit of 100psi
- Statistical Evaluation

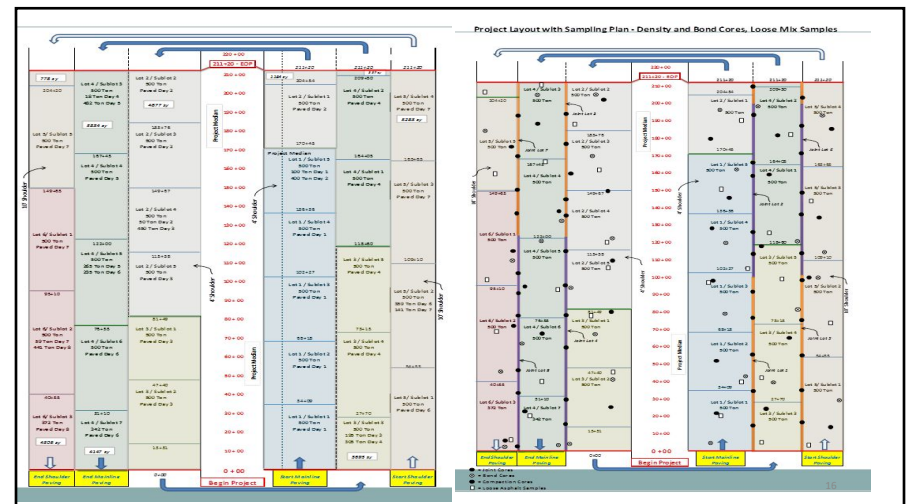


How Do We Know Where to Get These Samples

- Prior to Construction
- Lot Layout
 - Production lots are 2500 tons
 - Constructed joints have 10,000' lots
- Pre-paving Meeting
 - Agree on paving sequence – Layout Lots in field for sampling
 - Loose mix locations at beginning of day
 - Cores marked after finish roller...notify contractor!



15



PWL Paving – Example Setup

- Two lane, 3.0 mile road, 1.5 inch design thickness, 12 ft lanes. Example mix is 156.7 lb/CF max. How many tons?

Determine	Method	Answer
Thickness (ft)	Thickness / 12	
Length (ft)	Length(mi) x 5280	
Area (SF)	Length x Width x Lanes	
Volume (CF)	Area x Thickness	
Field Density	Max Density x 94%	
Pounds required	Field Density x Volume	
Tons required	Pounds / 2000	

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PWL Paving – Example Setup

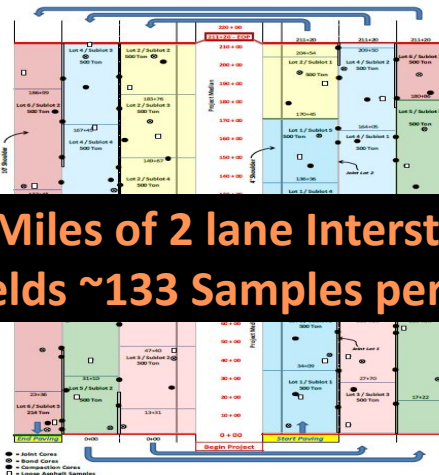
- 3500 ton project, single “extended” lot
- Sublot = 500 ton.... Therefore $3500/500 = 7$

Determine	Method	Answer
Loose Samples	One per mat subplot	
Mat Density Samples	One per mat subplot	
Bond Strength Samples	One per mat subplot	
Length (ft)	Length(mi) x 5280	
Joint Density Samples	One per JOINT subplot	
Thickness Samples	Use Bond Strength & Mat Density Cores	

18



**4 Miles of 2 lane Interstate
Yields ~133 Samples per lift**



19



Questions to ask before we start paving...

- What type of road is this?
- What will affect the PWL on my project?
- How can I help get the best PWL?

...on your “MARK”...get set...

PAVE!!

Significant Changes Field Evaluations – Pavement Distresses

MP 401.07.25
February, 2013

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
MATERIALS CONTROL, SOILS AND TESTING DIVISION
MATERIALS PROCEDURE

GUIDE FOR EVALUATION OF ASPHALT PAVEMENTS WITH SUBSTANDARD PROPERTIES

- PURPOSE**
 - 1.1 Provide a consistent approach and checklist for use by construction project personnel when evaluating asphalt pavement with substandard properties and aid in any subsequent decision.
 - 1.2 Identify pavement factors and characteristics most critical to satisfactory performance.
 - 1.3 Validate, if it is appropriate, the removal of the pavement in question.
- SCOPE**
 - 2.1 This procedure shall be applicable to all newly placed Marshall and Superpave mix base layers and wearing courses.
 - 2.2 All facets of construction including quality control, quality assurance, and independent assurance sampling and testing, along with construction practices and methods, and observable distresses and defects in the finished mix shall be considered when determining any action required to remediate the newly placed asphalt (course(s)). It is generally necessary to consider all facets when trying to determine the cause of substandard properties and observed distresses, and then decide on remedial action that needs to be executed.
- REFERENCED DOCUMENTS**
 - 3.1 Special Provision Section 401, Asphaltic Base, Wearing, and Patching and Levelling Courses, original issuance February 2013.



The Fork in the Road (New in 2023)

Interstates and Divided NHS Routes

- PWL Calculations on:
 - Mat Density
 - Joint Density
 - Bond Strength
 - Asphalt Content
 - #200(Dust) Content
- Lot Average
 - Thickness of cores

Commonalities:

- Main Lot Payment is always based on Mat Density, Asphalt Content, and Dust Content. With the Total Lot payment calculated the same regardless of how the Payment Factors are generated
 - 50% Mat Density, 25% each Asphalt content and Dust content
- Thickness, Bond Strength and Joint Density are each stand alone adjustments based on their individual equations

Two-Lane and Non-divided NHS Route Exceptions

- PWL Calculations on:
 - Asphalt Content
 - #200(Dust) Content
- Lot Average on:
 - Mat Density
 - Joint Density
 - Bond Strength
 - Thickness of Cores



22

PWL Paving – Pay Adjustment

• Lot Payment

$$CP(2PD + PB + PA)/400$$

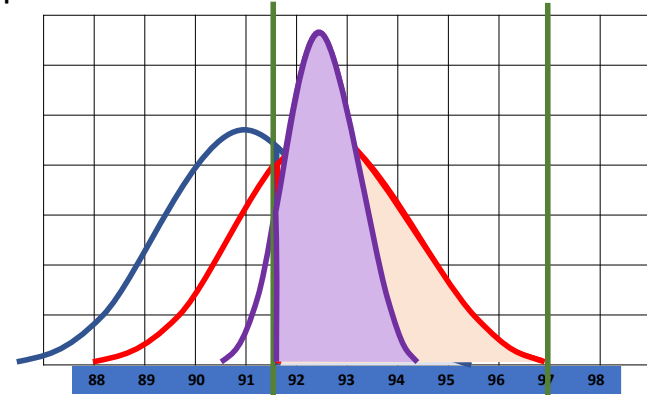
- Contract Price, Pay Density, Pay Binder, Pay Aggregate
- Payment for each component can reach 102%, but the average density must be greater than 93.0%



Incentives have been removed from the specification on projects let on of after January 1st, 2023.

23

Recap... what does PWL mean?



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PWL Paving – Example

• Mat Density (91.5 – 97.0)

• Example 1:

• 7 Cores - 93.00, 93.10, 92.30, 93.00, 93.50, 92.40, 92.60

• Avg = 92.84 Stdev = .43

• PWL = 100

• Example 2:

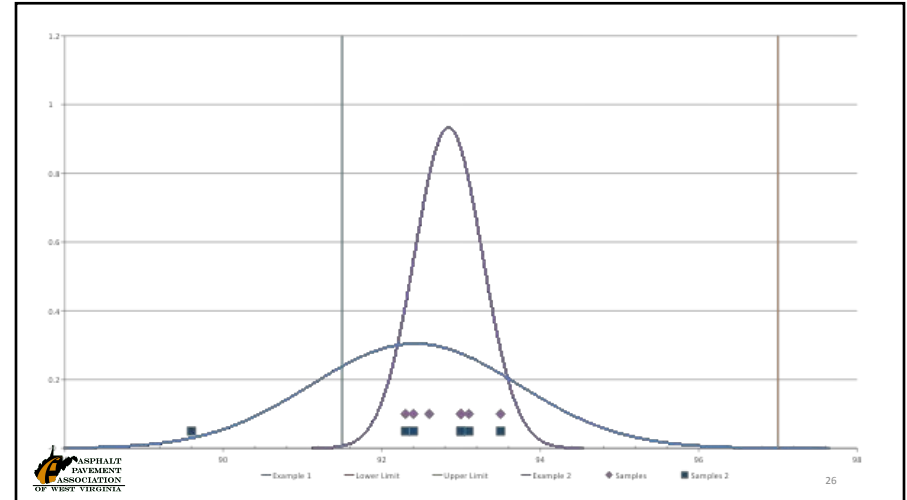
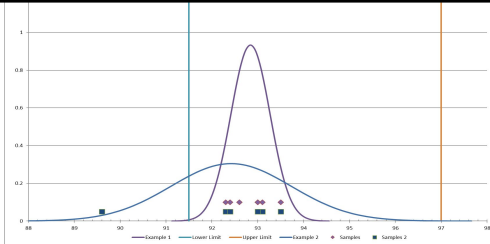
• 7 Cores - 93.00, 93.10, 92.30, 93.00, 93.50, 92.40, **89.60**

• Avg = 92.42 Stdev = 1.31

• PWL = 75



25



PWL Paving – Pay Adjustment

Interstates and Divided NHS Routes

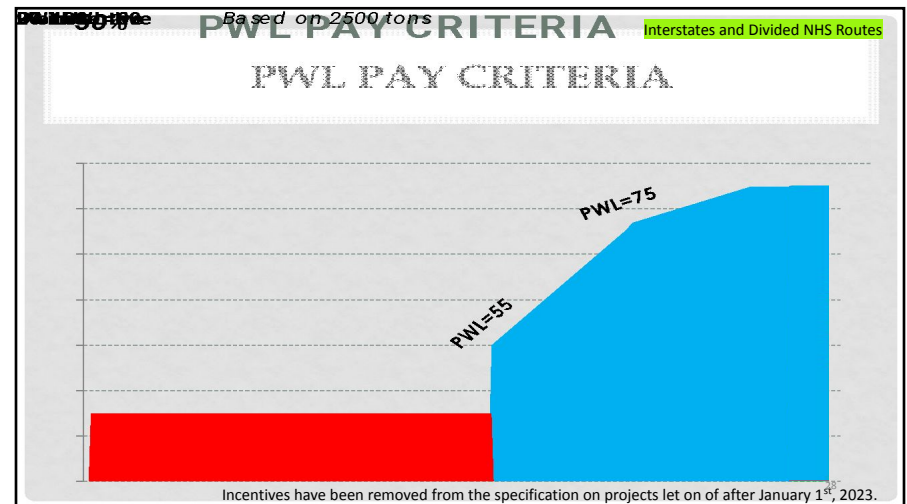
TABLE 410.13.3.1

Percentage of Material Within Specification Limits (PWL)	Lot Pay Factor (Percent of Contract Unit Price)
90-100	100
75-89	$[(0.5)PWL] + 55$
55-74 ^{Note 1}	$[(1.4)PWL] - 12$



Jobs let prior to January 1st, 2023 are still eligible for an incentives up to 2% for a PWL between 96-100

27



PWL Paving – Pay Adjustment

• Lot Payment

$$CP(2PD + PB + PA)/400$$

- Contract Price, Pay Density, Pay Binder, Pay Aggregate
- Payment for each component can reach 102%, but the average density must be greater than 93.0%



Incentives have been removed from the specification on projects let on or after January 1st, 2023.

29

Interstates and Divided NHS Routes

PWL Paving – Pay Adjustment Example

• Determine the pay factors for this example lot

Property	PWL	Pay Factor
Density	87	
Asphalt Content	90	
Gradation	86	

• What is the Lot payment

$$CP(2PD + PB + PA)/400$$



TABLE 410.13.3.1

Percentage of Material Within Specification Limits (PWL)	Lot Pay Factor (Percent of Contract Unit Price)
90-100	100
75-89	[(0.5)PWL]+55
55-74 ^{Note 1}	[(1.4)PWL]+42

PWL Paving – Pay Adjustment

• Thickness Payment

$$\% \text{ Adjustment} = \frac{t}{T} \times 100$$

- T = Total Plan Thickness
- t = average lot thickness + 0.04



31

Interstates and Divided NHS Routes

PWL Paving – Pay Adjustment

• Joint Payment

- When PWL > 60 there is no adjustment

$$\text{Disincentive}(\$) = \frac{60 - \text{PWL}}{60} \times 12,500 \quad (\text{When } \leq 60)$$

Jobs let prior to January 1st, 2023 are still eligible for up to a \$4000 incentives for PWL ≥ 80

$$\text{Incentive}(\$) = \frac{\text{PWL} - 80}{20} \times 4000$$



32

JOINT DENSITY PAY CRITERIA (TARGET= 89%)



PWL Paving – Pay Adjustment Example

- Joint Density PWL is 57. What is the Adjustment?



PWL Paving – Pay Adjustment Example

- Thickness target is 2 inches, and average is 1.92 inches. What is the pay adjustment?

$$\% \text{ Adjustment} = \frac{t}{T} \times 100$$

$$\% \text{ Adjustment Of Unit Price} = \quad \times 100$$

$$\% \text{ Adjustment Of Unit Price} =$$

- Unit price = \$10/sqyd
- Therefore an **Additional Standalone** Adjustment for inadequate thickness
 - Lot Area*(100-98%)/100*Unit Cost = 42,240*(.02)*\$10 = \$8448 Penalty



The Fork in the Road (New in 2023)

Interstates and Divided NHS Routes

- PWL Calculations on:

- Mat Density
- Joint Density
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- #200(Dust) Content

- Lot Average

- Thickness of cores

Commonalities:

- Main Lot Payment is always based on Mat Density, Asphalt Content, and Dust Content. With the Total Lot payment calculated the same regardless of how the Payment Factors are generated
 - 50% Mat Density, 25% each Asphalt content and Dust content

- Thickness, Bond Strength and Joint Density are each stand alone adjustments based on their individual equations



Two Lane Exceptions

- AC, Gradation, and Thickness are still calculated the same as Interstate PWL
- Mat Density, Joint Density, and Bond Strength are calculated based on the lot average



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Mat Density PD

TABLE 410.13.7.2

Payment Factors for Pavement Mat Density	
Average Lot Percent Density	Payment Factor (PD)
Greater than 98 %	Note 1
91.50% to 98.00%	100
88.00% to 91.49%	$= 100 - 4*(91.50\% - \text{Percent density})$
Less than 88%	$= 84 - 10*(88\% - \text{Percent density})$ ^{Note 2}

Example 1:

7 Cores - 93.00, 93.10, 92.30, 93.00, 93.50, 92.40, 92.60

Avg = 92.84 Stdev = .43

PWL=100 → PD = 100

Example 2:

7 Cores - 93.00, 93.10, 92.30, 93.00, 93.50, 92.40, **89.60**

Avg = 92.42 Stdev = 1.31

PD = 100



38

Joint Density Adjustment

TABLE 410.13.7.3

Pay Adjustment for Pavement Joint Density per Linear Foot	
Average Lot Percent Density	Price Adjustment (\$ / LF)
Greater than 97 %	Note 3
89.00 % to 97.00%	0
88.00% to 88.99%	-0.20
Less than 88%	$= [0.50*(\text{Percent Density} - 88.00)] - 0.20$



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Bond Strength Adjustment

TABLE 410.13.7.4

Pay Adjustment for Bond Strength per 2500 Ton Lot	
Average Lot Bond Strength (PSI)	Price Adjustment (\$ / Lot)
100.00 and Greater	0
75.00 to 99.99	$= [1,000*(\text{PSI} - 75.00)] - 25,000$
Less than 75.00	- 25,000



40

Confused??



PWL boils down to controlling variability

41

Let's tie this back to yesterday... Understanding the Variability

- Material Variations
- Testing Variations
- Plant Operations
- Environmental Conditions
- Delivery of Material
- Field Prep
- Paving Operations
- Rolling Operations



42

Materials and Testing Variations

- Aggregates
 - Stockpiles Gradation
 - Aggregate Texture
 - Daily if not multiple daily moisture checks
 - Stockpile size

• Paving



Plant Operations

- Material Feed
- Bag house Control
 - Dust Creation
- Liquid and belt Calibration
- Moisture Control
- Temperature Control
- Starts and Stops/Job Switching
 - Material waste
- Inspection – Plant wear



44



Field Preparation

- Milling Profile
- Tack Coats

Delivery of Material

- Insulated Trucks
- Sound Tarps on Trucks
- Constant Material Delivery
- Haul distance
- Use of an MTV

Paving Operations

- Paver Setup
- Auger Extension
- Tunnel Extension

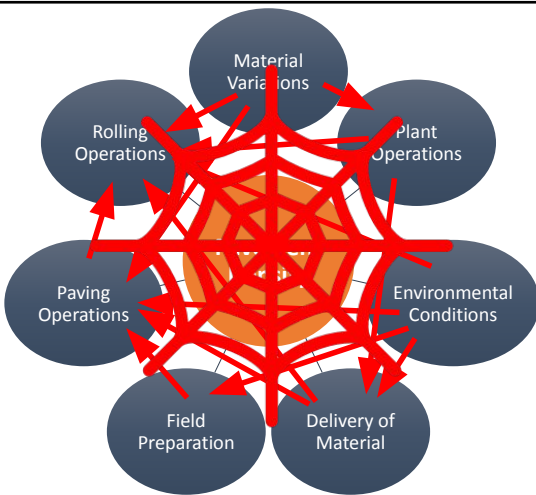
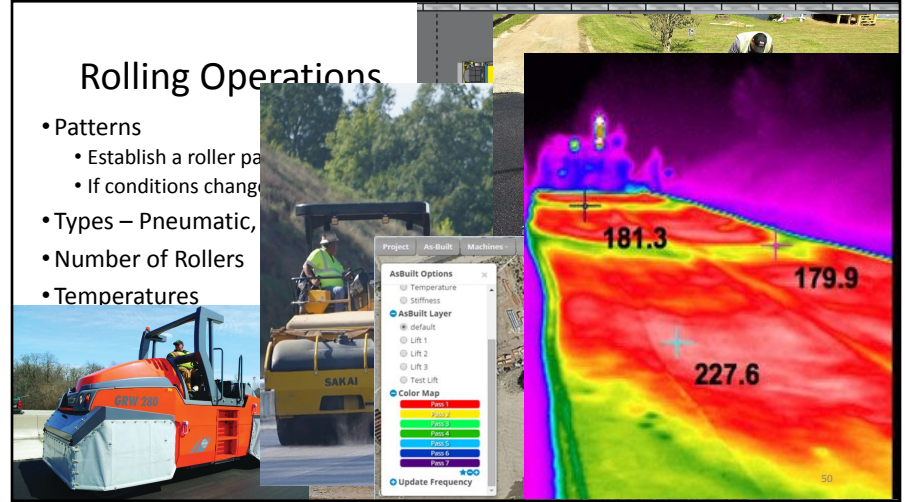
Paving Operations

- Constant Paver Speed throughout the day
 - DO NOT STOP
 - Constant Material Delivery
- Thickness of the Lift
 - Consistent Lift = Consistent Compaction



Rolling Operations

- Patterns
 - Establish a roller pattern
 - If conditions change
- Types – Pneumatic,
- Number of Rollers
- Temperatures

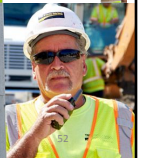
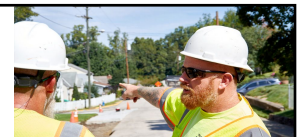


51

Communication



- Everyone needs to be involved/ Communicate with everyone Involved
 - Plant Personnel – especially your loader operator
 - Lab Tech and field Tech
 - Truckers
 - Roller Operators
 - Paving Operators
- Sample Data needs to be communicated to the project personnel

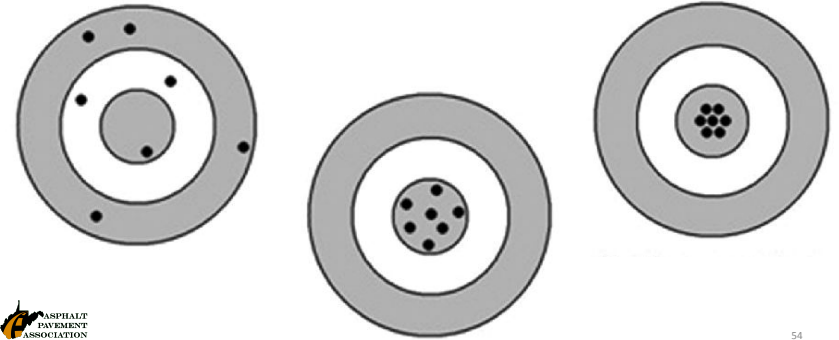


WHY YOU SHOULD CARE ABOUT THIS!!!



53

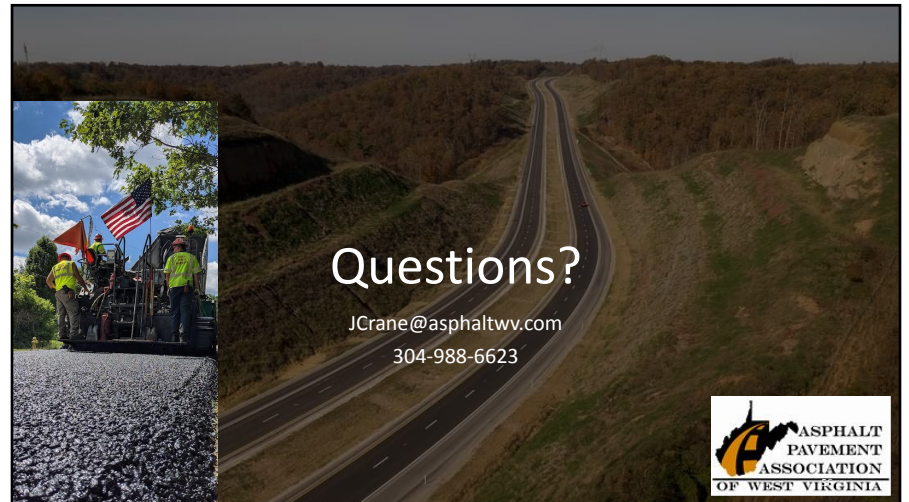
Remember it is all about CONSISTENCY



54



55



Questions?

JCrane@asphaltwv.com

304-988-6623





HMA Construction Program

Module 10 Troubleshooting

7-1



Learning Objectives

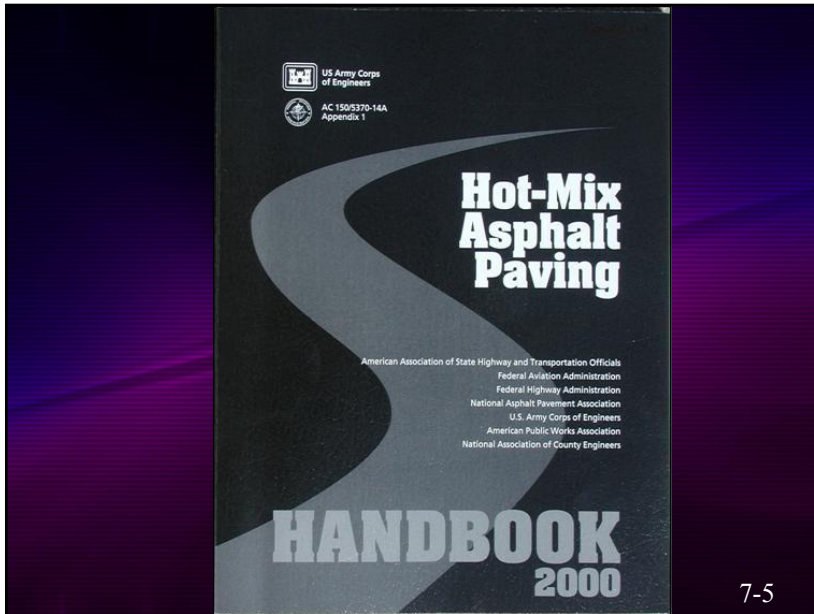
1. State the objective of effective troubleshooting
2. List the steps needed to effectively troubleshoot
3. Analyze situations and recommend action to be taken

7-3

Objective of Effective Troubleshooting?

- Isolating the problem so that an appropriate solution can be arrived at quickly and efficiently.

7-4



MAT PROBLEM TROUBLE-SHOOTING GUIDE

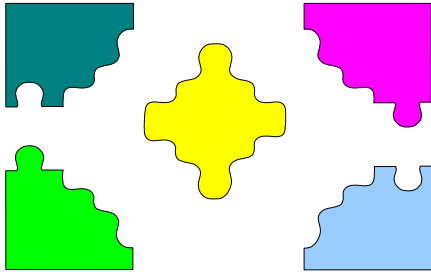
PG 195

CAUSES

PROBLEM	1. Poor Subgrade	2. Poor Mix	3. Poor Compaction	4. Poor Timing	5. Poor Weather	6. Poor Equipment	7. Poor Operator	8. Poor Maintenance	9. Poor Materials	10. Poor Design	11. Poor Construction	12. Poor Supervision	13. Poor Communication	14. Poor Coordination	15. Poor Planning	16. Poor Organization	17. Poor Management	18. Poor Leadership	19. Poor Teamwork	20. Poor Accountability	21. Poor Responsibility	22. Poor Commitment	23. Poor Motivation	24. Poor Incentives	25. Poor Constraints	26. Poor Resources	27. Poor Information	28. Poor Knowledge	29. Poor Skills	30. Poor Attitudes	31. Poor Values	32. Poor Beliefs	33. Poor Assumptions	34. Poor Expectations	35. Poor Goals	36. Poor Objectives	37. Poor Strategies	38. Poor Tactics	39. Poor Procedures	40. Poor Policies	41. Poor Standards	42. Poor Metrics	43. Poor Benchmarks	44. Poor Best Practices	45. Poor Innovation	46. Poor Creativity	47. Poor Problem Solving	48. Poor Decision Making	49. Poor Risk Management	50. Poor Quality Management	51. Poor Safety Management	52. Poor Environmental Management	53. Poor Social Management	54. Poor Governance	55. Poor Leadership	56. Poor Teamwork	57. Poor Accountability	58. Poor Responsibility	59. Poor Commitment	60. Poor Motivation	61. Poor Incentives	62. Poor Constraints	63. Poor Resources	64. Poor Information	65. Poor Knowledge	66. Poor Skills	67. Poor Attitudes	68. Poor Values	69. Poor Beliefs	70. Poor Assumptions	71. Poor Expectations	72. Poor Goals	73. Poor Objectives	74. Poor Strategies	75. Poor Tactics	76. Poor Procedures	77. Poor Policies	78. Poor Standards	79. Poor Metrics	80. Poor Benchmarks	81. Poor Best Practices	82. Poor Innovation	83. Poor Creativity	84. Poor Problem Solving	85. Poor Decision Making	86. Poor Risk Management	87. Poor Quality Management	88. Poor Safety Management	89. Poor Environmental Management	90. Poor Social Management	91. Poor Governance	92. Poor Leadership	93. Poor Teamwork	94. Poor Accountability	95. Poor Responsibility	96. Poor Commitment	97. 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Poor Goals	369. Poor Objectives	370. Poor Strategies	371. Poor Tactics	372. Poor Procedures	373. Poor Policies	374. Poor Standards	375. Poor Metrics	376. Poor Benchmarks	377. Poor Best Practices	378. Poor Innovation	379. Poor Creativity	380. Poor Problem Solving	381. Poor Decision Making	382. Poor Risk Management	383. Poor Quality Management	384. Poor Safety Management	385. Poor Environmental Management	386. Poor Social Management	387. Poor Governance	388. Poor Leadership	389. Poor Teamwork	390. Poor Accountability	391. Poor Responsibility	392. Poor Commitment	393. Poor Motivation	394. Poor Incentives	395. Poor Constraints	396. Poor Resources	397. Poor Information	398. Poor Knowledge	399. Poor Skills	400. Poor Attitudes	401. Poor Values	402. Poor Beliefs	403. Poor Assumptions	404. Poor Expectations	405. Poor Goals	406. Poor Objectives	407. Poor Strategies	408. Poor Tactics	409. Poor Procedures	410. Poor Policies	411. Poor Standards	412. Poor Metrics	413. Poor Benchmarks	414. Poor Best Practices	415. Poor Innovation	416. Poor Creativity	417. Poor Problem Solving	418. Poor Decision Making	419. Poor Risk Management	420. Poor Quality Management	421. Poor Safety Management	422. Poor Environmental Management	423. Poor Social Management	424. Poor Governance	425. Poor Leadership	426. Poor Teamwork	427. Poor Accountability	428. Poor Responsibility	429. Poor Commitment	430. Poor Motivation	431. Poor Incentives	432. Poor Constraints	433. Poor Resources	434. Poor Information	435. Poor Knowledge	436. Poor Skills	437. Poor Attitudes	438. Poor Values	439. Poor Beliefs	440. Poor Assumptions	441. Poor Expectations	442. Poor Goals	443. Poor Objectives	444. Poor Strategies	445. Poor Tactics	446. Poor Procedures	447. Poor Policies	448. Poor Standards	449. Poor Metrics	450. Poor Benchmarks	451. Poor Best Practices	452. Poor Innovation	453. Poor Creativity	454. Poor Problem Solving	455. Poor Decision Making	456. Poor Risk Management	457. Poor Quality Management	458. Poor Safety Management	459. Poor Environmental Management	460. Poor Social Management	461. Poor Governance	462. Poor Leadership	463. Poor Teamwork	464. Poor Accountability	465. Poor Responsibility	466. Poor Commitment	467. Poor Motivation	468. Poor Incentives	469. Poor Constraints	470. Poor Resources	471. Poor Information	472. Poor Knowledge	473. Poor Skills	474. Poor Attitudes	475. Poor Values	476. Poor Beliefs	477. Poor Assumptions	478. Poor Expectations	479. Poor Goals	480. Poor Objectives	481. Poor Strategies	482. Poor Tactics	483. Poor Procedures	484. Poor Policies	485. Poor Standards	486. Poor Metrics	487. Poor Benchmarks	488. Poor Best Practices	489. Poor Innovation	490. Poor Creativity	491. Poor Problem Solving	492. Poor Decision Making	493. Poor Risk Management	494. Poor Quality Management	495. Poor Safety Management	496. Poor Environmental Management	497. Poor Social Management	498. Poor Governance	499. Poor Leadership	500. Poor Teamwork	501. Poor Accountability	502. Poor Responsibility	503. Poor Commitment	504. Poor Motivation	505. Poor Incentives	506. Poor Constraints	507. Poor Resources	508. Poor Information	509. Poor Knowledge	510. Poor Skills	511. Poor Attitudes	512. Poor Values	513. Poor Beliefs	514. Poor Assumptions	515. Poor Expectations	516. Poor Goals	517. Poor Objectives	518. Poor Strategies	519. Poor Tactics	520. Poor Procedures	521. Poor Policies	522. Poor Standards	523. Poor Metrics	524. Poor Benchmarks	525. Poor Best Practices	526. Poor Innovation	527. Poor Creativity	528. Poor Problem Solving	529. Poor Decision Making	530. Poor Risk Management	531. Poor Quality Management	532. Poor Safety Management	533. Poor Environmental Management	534. Poor Social Management	535. Poor Governance	536. Poor Leadership	537. Poor Teamwork	538. Poor Accountability	539. Poor Responsibility	540. Poor Commitment	541. Poor Motivation	542. Poor Incentives	543. Poor Constraints	544. Poor Resources	545. Poor Information	546. Poor Knowledge	547. Poor Skills	548. Poor Attitudes	549. Poor Values	550. Poor Beliefs	551. Poor Assumptions	552. Poor Expectations	553. Poor Goals	554. Poor Objectives	555. Poor Strategies	556. Poor Tactics	557. Poor Procedures	558. Poor Policies	559. Poor Standards	560. Poor Metrics	561. Poor Benchmarks	562. Poor Best Practices	563. Poor Innovation	564. Poor Creativity	565. Poor Problem Solving	566. Poor Decision Making	567. Poor Risk Management	568. Poor Quality Management	569. Poor Safety Management	570. Poor Environmental Management	571. Poor Social Management	572. Poor Governance	573. Poor Leadership	574. Poor Teamwork	575. Poor Accountability	576. Poor Responsibility	577. Poor Commitment	578. Poor Motivation	579. Poor Incentives	580. Poor Constraints	581. Poor Resources	582. Poor Information	583. Poor Knowledge	584. Poor Skills	585. Poor Attitudes	586. Poor Values	587. Poor Beliefs	588. Poor Assumptions	589. Poor Expectations	590. Poor Goals	591. Poor Objectives	592. Poor Strategies	593. Poor Tactics	594. Poor Procedures	595. Poor Policies	596. Poor Standards	597. Poor Metrics	598. Poor Benchmarks	599. Poor Best Practices	600. Poor Innovation	601. Poor Creativity	602. Poor Problem Solving	603. Poor Decision Making	604. Poor Risk Management	605. Poor Quality Management	606. Poor Safety Management	607. Poor Environmental Management	608. Poor Social Management	609. Poor Governance	610. Poor Leadership	611. Poor Teamwork	612. Poor Accountability	613. Poor Responsibility	614. Poor Commitment	615. Poor Motivation	616. Poor Incentives	617. Poor Constraints	618. Poor Resources	619. Poor Information	620. Poor Knowledge	621. Poor Skills	622. Poor Attitudes	623. Poor Values	624. Poor Beliefs	625. Poor Assumptions	626. Poor Expectations	627. Poor Goals	628. Poor Objectives	629. Poor Strategies	630. Poor Tactics	631. Poor Procedures	632. Poor Policies	633. Poor Standards	634. Poor Metrics	635. Poor Benchmarks	636. Poor Best Practices	637. Poor Innovation	638. Poor Creativity	639. Poor Problem Solving	640. Poor Decision Making	641. Poor Risk Management	642. Poor Quality Management	643. Poor Safety Management	644. Poor Environmental Management	645. Poor Social Management	646. Poor Governance	647. Poor Leadership	648. Poor Teamwork	649. Poor Accountability	650. Poor Responsibility	651. Poor Commitment	652. Poor Motivation	653. Poor Incentives	654. Poor Constraints	655. Poor Resources	656. Poor Information	657. Poor Knowledge	658. Poor Skills	659. Poor Attitudes	660. Poor Values	661. Poor Beliefs	662. Poor Assumptions	663. Poor Expectations	664. Poor Goals	665. Poor Objectives	666. Poor Strategies	667. Poor Tactics	668. Poor Procedures	669. Poor Policies	670. Poor Standards	671. Poor Metrics	672. Poor Benchmarks	673. Poor Best Practices	674. Poor Innovation	675. Poor Creativity	676. Poor Problem Solving	677. Poor Decision Making	678. Poor Risk Management	679. Poor Quality Management	680. Poor Safety Management	681. Poor Environmental Management	682. Poor Social Management	683. Poor Governance	684. Poor Leadership	685. Poor Teamwork	686. Poor Accountability	687. Poor Responsibility	688. Poor Commitment	689. Poor Motivation	690. Poor Incentives	691. Poor Constraints	692. Poor Resources	693. Poor Information	694. Poor Knowledge	695. Poor Skills	696. Poor Attitudes	697. Poor Values	698. Poor Beliefs	699. Poor Assumptions	700. Poor Expectations	701. Poor Goals	702. Poor Objectives	703. Poor Strategies	704. Poor Tactics	705. Poor Procedures	706. Poor Policies	707. Poor Standards	708. Poor Metrics	709. Poor Benchmarks	710. Poor Best Practices	711. Poor Innovation	712. Poor Creativity	713. Poor Problem Solving	714. Poor Decision Making	715. Poor Risk Management	716. Poor Quality Management	717. Poor Safety Management	718. Poor Environmental Management	719. Poor Social Management	720. Poor Governance	721. Poor Leadership	722. Poor Teamwork	723. Poor Accountability	724. Poor Responsibility	725. Poor Commitment	726. Poor Motivation	727. 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Poor Assumptions	774. Poor Expectations	775. Poor Goals	776. Poor Objectives	777. Poor Strategies	778. Poor Tactics	779. Poor Procedures	780. Poor Policies	781. Poor Standards	782. Poor Metrics	783. Poor Benchmarks	784. Poor Best Practices	785. Poor Innovation	786. Poor Creativity	787. Poor Problem Solving	788. Poor Decision Making	789. Poor Risk Management	790. Poor Quality Management	791. Poor Safety Management	792. Poor Environmental Management	793. Poor Social Management	794. Poor Governance	795. Poor Leadership	796. Poor Teamwork	797. Poor Accountability	798. Poor Responsibility	799. Poor Commitment	800. Poor Motivation	801. Poor Incentives	802. Poor Constraints	803. Poor Resources	804. Poor Information	805. Poor Knowledge	806. Poor Skills	807. Poor Attitudes	808. Poor Values	809. Poor Beliefs	810. Poor Assumptions	811. Poor Expectations	812. Poor Goals	813. Poor Objectives	814. Poor Strategies	815. Poor Tactics	816. Poor Procedures	817. Poor Policies	818. Poor Standards	819. Poor Metrics	820. Poor Benchmarks	821. Poor Best Practices	822. Poor Innovation	823. Poor Creativity	824. Poor Problem Solving	825. Poor Decision Making	826. Poor Risk Management	827. Poor Quality Management	828. Poor Safety Management	829. Poor Environmental Management	830. Poor Social Management	831. Poor Governance	832. Poor Leadership	833. Poor Teamwork	834. Poor Accountability	835. Poor Responsibility	836. Poor Commitment	837. Poor Motivation	838. Poor Incentives	839. Poor Constraints	840. Poor Resources	841. Poor Information	842. Poor Knowledge	843. Poor Skills	844. Poor Attitudes	845. Poor Values	846. Poor Beliefs	847. Poor Assumptions	848. Poor Expectations	849. Poor Goals	850. Poor Objectives	851. Poor Strategies	852. Poor Tactics	853. Poor Procedures	854. Poor Policies	855. Poor Standards	856. Poor Metrics	857. Poor Benchmarks	858. Poor Best Practices	859. Poor Innovation	860. Poor Creativity	861. Poor Problem Solving	862. Poor Decision Making	863. Poor Risk Management	864. Poor Quality Management	865. Poor Safety Management	866. Poor Environmental Management	867. Poor Social Management	868. Poor Governance	869. Poor Leadership	870. Poor Teamwork	871. Poor Accountability	872. Poor Responsibility	873. Poor Commitment	874. Poor Motivation	875. Poor Incentives	876. Poor Constraints	877. Poor Resources	878. Poor Information	879. Poor Knowledge	880. Poor Skills	881. Poor Attitudes	882. Poor Values	883. Poor Beliefs	884. Poor Assumptions	885. Poor Expectations	886. Poor Goals	887. Poor Objectives	888. Poor Strategies	889. Poor Tactics	890. Poor Procedures	891. Poor Policies	892. Poor Standards	893. Poor Metrics	894. Poor Benchmarks	895. Poor Best Practices	896. Poor Innovation	897. Poor Creativity	898. Poor Problem Solving	899. Poor Decision Making	900. Poor Risk Management	901. Poor Quality Management	902. Poor Safety Management	903. Poor Environmental Management	904. Poor Social Management	905. Poor Governance	906. Poor Leadership	907. Poor Teamwork	908. Poor Accountability	909. Poor Responsibility	910. Poor Commitment	911. Poor Motivation	912. Poor Incentives	913. Poor Constraints	914. Poor Resources	915. Poor Information	916. Poor Knowledge	917. Poor Skills	918. Poor Attitudes	919. Poor Values	920. Poor Beliefs	921. Poor Assumptions	922. Poor Expectations	923. Poor Goals	924. Poor Objectives	925. Poor Strategies	926. Poor Tactics	927. Poor Procedures	928. Poor Policies	929. Poor Standards	930. Poor Metrics	931. Poor Benchmarks	932. Poor Best Practices	933. Poor Innovation	934. Poor Creativity	935. Poor Problem Solving	936. Poor Decision Making	937. Poor Risk Management	938. Poor Quality Management	939. Poor Safety Management	940. Poor Environmental Management	941. Poor Social Management	942. Poor Governance	943. Poor Leadership	944. Poor Teamwork	945. Poor Accountability	946. Poor Responsibility	947. Poor Commitment	948. Poor Motivation	949. Poor Incentives	950. Poor Constraints	951. Poor Resources	952. Poor Information	953. 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How to Troubleshoot

- Step Two:
Break the problem down into pieces



7-9

How to Troubleshoot

- Step Three:
Eliminate the obvious factors first

7-10

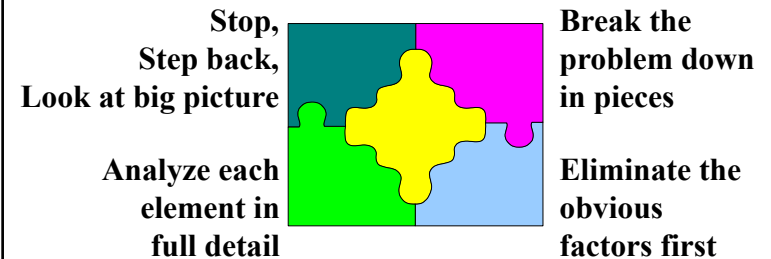
How to Troubleshoot

- Step Four:
Analyze each remaining element in full detail

7-11

How to Troubleshoot

- Step Five:
Re-combine the pieces of puzzle



7-12

How to Troubleshoot

- Step Six:

Make recommendations based on facts

7-13

How to Troubleshoot

- Step Seven:

Make changes “one at a time;” then analyze results

7-14

How to Troubleshoot

- Step Eight:

Take notes every step of the way



7-15

How to Troubleshoot



Problem	Causes																		
	Variable of temperature of concrete	Moisture in Mix	Moisture in Mat	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade
Wavy Surface – Short Waves (Ripples)	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Wavy Surface – Long Waves	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Tearing of Mat – Full Width	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Tearing of Mat – Center Streaks	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Tearing of Mat – Outside Streaks	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Mat Texture Nonuniform	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Screed Marks	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Screed Not Responding to correction	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Auger Shadows	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Poor Percompaction	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Poor Longitudinal Joint	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Poor Transverse Joint	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Transverse Cracking (Checking)	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Mat Shoving Under Roller	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Bleeding or Fat Spots in Mat	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Roller Marks	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√
Poor Mix Compaction	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√	√

√ Indicates a paver problem, x indicates a problem to be investigated

There can be multiple causes of problems, investigate each one.

9-17

7-17



01/01/2002



01/01/2002

Problem	Causes									
	Cold Mix Temperature	Moisture in Mix	Moisture in Mat	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade	Moisture in Subgrade
Wavy Surface – Short Waves (Ripples)	x	x	x	x	x	x	x	x	x	x
Wavy Surface – Long Waves	x	x	x	x	x	x	x	x	x	x
Tearing of Mat – Full Width	x	x	x	x	x	x	x	x	x	x
Tearing of Mat – Center Streaks	x	x	x	x	x	x	x	x	x	x
Tearing of Mat – Outside Streaks	x	x	x	x	x	x	x	x	x	x
Mat Texture Nonuniform	x	x	x	x	x	x	x	x	x	x
Screed Marks	x	x	x	x	x	x	x	x	x	x
Screed Not Responding to correction	x	x	x	x	x	x	x	x	x	x
Auger Shadows	x	x	x	x	x	x	x	x	x	x
Poor Percompaction	x	x	x	x	x	x	x	x	x	x
Poor Longitudinal Joint	x	x	x	x	x	x	x	x	x	x
Poor Transverse Joint	x	x	x	x	x	x	x	x	x	x
Transverse Cracking (Checking)	x	x	x	x	x	x	x	x	x	x
Mat Shoving Under Roller	x	x	x	x	x	x	x	x	x	x
Bleeding or Fat Spots in Mat	x	x	x	x	x	x	x	x	x	x
Roller Marks	x	x	x	x	x	x	x	x	x	x
Poor Mix Compaction	x	x	x	x	x	x	x	x	x	x

√ Indicates a paver problem, x indicates a problem to be investigated

There can be multiple causes of problems, investigate each one.

7-20

7-20

Segregation Troubleshooting

That's 18 possible causes for non-uniform mat texture!

7-21

Segregation Troubleshooting

That's 18 possible causes for non-uniform mat texture!

7-21

Segregation Troubleshooting

That's 18 possible causes for non-uniform mat texture!

7-21

How to Troubleshoot

- Stop, step back, look at big picture
 - Break the problem down in pieces
 - Eliminate the obvious factors first
 - Analyze each element in full detail
 - Re-combine the pieces of puzzle
 - Recommendation based on facts
- Changes “one at a time;” analyze
- Take notes every step of the way

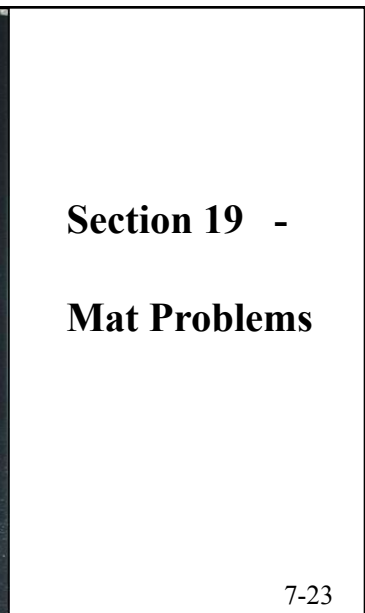
7-22

- ## How to Troubleshoot
- Stop, step back, look at big picture
 - Break the problem down in pieces
 - Eliminate the obvious factors first
 - Analyze each element in full detail
 - Re-combine the pieces of puzzle
 - Recommendation based on facts
 - Changes “one at a time;” analyze
 - Take notes every step of the way
- 7-22

How to Troubleshoot

- Stop, step back, look at big picture
 - Break the problem down in pieces
 - Eliminate the obvious factors first
 - Analyze each element in full detail
 - Re-combine the pieces of puzzle
 - Recommendation based on facts
- Changes “one at a time;” analyze
- Take notes every step of the way

7-22



Section 19 -

Mat Problems

7-23

Section 19 -

Mat Problems

7-23

Problem	Causes												
	Cold Mix Temperature Variation of Temperature of Mix Moisture in Mix Mix Segregation Improper Mix Design (Depth) Improper Mix Design (Aggregate) Pulling of Rollers on Hot Mix Forwarding of too Rapidly Turning Rollers Improper Roller Operation Improper Base Preparation Trucks Holding Brakes Trucks Bumping Finisher Improper Mix Thickness for Aggregate Size Improper Joint Overlap Sliding Long Paved Between Lanes Grade Reference Inadequate Grade Control Water Bouncing on Reference Grade Control Timing (Scheduling too High) Grade Control Nocturnal Inaccuracy Weather Running too Slow Speed Emission Instability Inaccuracy Screen Starting Block to too Short Incorrect Tilling of Screen Kicker Screws Warm Oil or Nocturnal Inaccuracy Road Cane Set Inaccuracy Running Tipped Empty Grinders Lacks Moisture on Shoulder too Low Cold Screen Speed Plates Not Tight Speed Plates Warm Oil or Waxed Screws Running on Hot Grinders Excessive Play in Screws Machine or Control On or Control of Machine and Control Screws Too Much Load on Screws Finisher Speed Too Fast Finisher Speed Too Overload Finishing time of material												
Wavy Surface – Short Waves (Ripples)	✓	✓											
Wavy Surface – Long Waves	✓	✓											
Tearing of Matt – Full Width		✓											
Tearing of Matt – Center Streaks			✓		✓	✓	✓						
Tearing of Matt – Outside Streaks			✓		✓	✓	✓						
Mat Texture Nonuniform	✓	✓	✓	✓	✓	✓	✓						
Scribed Marks				✓									
Screen Not Responding to correction		✓	✓	✓	✓			✓					
Auger Shadows		✓											
Poor Percompaction	✓		✓					✓					
Poor Longitudinal Joint	✓	✓		✓	✓	✓	✓	✓	✓	✓			
Poor Transverse Joint	✓		✓	✓	✓	✓		✓			✓		
Transverse Cracking (Checking)											✓	✓	✓
Mat Shoving Under Roller											✓	✓	✓
Bleeding or Fat Spots in Mat											✓	✓	✓
Roller Marks											✓	✓	✓
Poor Mix Compaction											✓	✓	✓

✓ Indicates a paver problem, × indicates a problem to be investigated

There can be multiple causes of problems, investigate each one.

7-24

Problem	Causes												
	Cold Mix Temperature Variation of Temperature of Mix Moisture in Mix Mix Segregation Improper Mix Design (Depth) Improper Mix Design (Aggregate) Pulling of Rollers on Hot Mix Forwarding of too Rapidly Turning Rollers Improper Roller Operation Improper Base Preparation Trucks Holding Brakes Trucks Bumping Finisher Improper Mix Thickness for Aggregate Size Improper Joint Overlap Sliding Long Paved Between Lanes Grade Reference Inadequate Grade Control Water Bouncing on Reference Grade Control Timing (Scheduling too High) Grade Control Nocturnal Inaccuracy Weather Running too Slow Speed Emission Instability Inaccuracy Screen Starting Block to too Short Incorrect Tilling of Screen Kicker Screws Warm Oil or Nocturnal Inaccuracy Road Cane Set Inaccuracy Running Tipped Empty Grinders Lacks Moisture on Shoulder too Low Cold Screen Speed Plates Not Tight Speed Plates Warm Oil or Waxed Screws Running on Hot Grinders Excessive Play in Screws Machine or Control On or Control of Machine and Control Screws Too Much Load on Screws Finisher Speed Too Fast Finisher Speed Too Overload Finishing time of material												
Wavy Surface – Short Waves (Ripples)	✓	✓											
Wavy Surface – Long Waves	✓	✓											
Tearing of Matt – Full Width		✓											
Tearing of Matt – Center Streaks			✓		✓	✓	✓						
Tearing of Matt – Outside Streaks			✓		✓	✓	✓						
Mat Texture Nonuniform	✓	✓	✓	✓	✓	✓	✓						
Scribed Marks				✓									
Screen Not Responding to correction		✓	✓	✓	✓			✓					
Auger Shadows		✓											
Poor Percompaction	✓		✓					✓					
Poor Longitudinal Joint	✓	✓		✓	✓	✓	✓	✓	✓	✓			
Poor Transverse Joint	✓		✓	✓	✓	✓		✓			✓		
Transverse Cracking (Checking)											✓	✓	✓
Mat Shoving Under Roller											✓	✓	✓
Bleeding or Fat Spots in Mat											✓	✓	✓
Roller Marks											✓	✓	✓
Poor Mix Compaction											✓	✓	✓

✓ Indicates a paver problem, × indicates a problem to be investigated

There can be multiple causes of problems, investigate each one.

7-24

Example 1

Your paving team has been using a double drum steel wheel roller as the breakdown roller on an 1 ½" of Wearing 1 overlay.

You are experiencing some checking in the mat after the second or third pass.

The mix has been tested at the lab and isn't suspect
– Asphalt & Dust contents are good.

7-25

Example 1



Example 1



photo courtesy of NCAT

Example 1

- What would cause this?

Handbook Pages 202 – 203

- What can you do right now to correct this?

Handbook Pages 203 - 204

7-28

Example 2

You are show up on the Project and see random fat spots and bleeding in the wheel paths on a 1" Wearing 1 overlay placed yesterday .

This is slightly more noticeable at the transverse joints.

The test results from the day before meets the specifications.

You are ready to start paving the same mix on the same stretch of road today.

7-29

Example 2



Example 2



Example 2

- What would cause this?
- Should you Pave today?
- What can you do right now to correct this?

7-32

Example 2



Example 3

A test strip of 19 mm Asphalt has been completed on a \$2 Million paving Project and you have been comparing roller passes to in-place density – measured with cores.

Even with a double drum vibratory and a 10 ton pneumatic roller, you are barely able to achieve the minimum density on the 2 ½" mat.

The Contractor and Agency are not in agreement of how to proceed.

Yes, the test strip did pass, but do you start paving?

7-34

Example 3

- As a consultant, what do you recommend?
- What all should you check before proceeding?
- What are your options?

7-35

Example 3



Page 155

Example 3



Example 3



Example 3



Example 4

You are paving a 9.5mm Skid wearing mix on a 2 lane road. As a compaction technician, you are struggling with compaction and notice that the surface is “spongy” at 130° F. You can twist your heel into the surface and make a sizable scar.

What could cause this, and what can you do?

Example 4



Example 4



Example 4



Example 4



Example 4



Example 4



Questions??

Disclaimer : Photos used within this presentation are for illustrative purposes only, any references to individuals or specific products are unintentional and coincidental. (CDM)

WVDOH

DENSITY EVALUATION

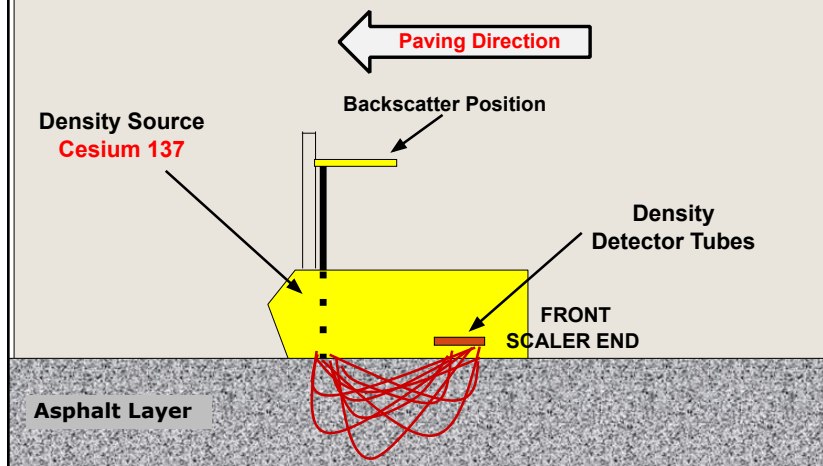
MCS&T - October 17, 2024

Jamie Rose - Quarry Operations Manager - J.F. Allen Company

Objectives :

- ❑ 401 Compaction Overview
- ❑ Gauge Comparisons
- ❑ Lot by Lot - Random Locations
- ❑ Roller Pass Method
- ❑ MP 401.05.20 - Deactivated in 2023
- ❑ Section 401 – 2023 Spec Book
- ❑ Section 401 – Supplemental Specifications

Troxler 3400 Series Nuclear Gauges



Standard Practices

Pre-Operation:

- ❑ Has Gauge been Calibrated within the last 24 Months
- ❑ Proper Bill Of Lading for Transport
- ❑ Is Gauge Properly Charged
- ❑ Proper Standard Block in Case
- ❑ Gauge and Standard Block Cleaned

Standard Practices

Operation – Standardizing the Gauge:

- Verify that the Standard Block and the bottom of the Gauge are clean.
- Place the Gauge on the Standard Block in the correct direction.
- Make sure Standard Block is sitting on a solid surface away from large metal objects such as equipment and trucks.
- Make sure you don't standardize within 30 feet of another gauge.
- Check to make sure Gauge Standard counts are within limits of the Calibrated Manufacturer's Standard Counts.
- **Density = +/- 2%**
- **Moisture = +/- 4%**

Standard Practices

Gauge Model: 3430
Serial Number: 35998
Calib. Date: 03/18/2020

Expires: 03/18/2021

Density Std. Cnt: 2066
Moisture Std. Cnt: 622
Bay Number: 1

Density
Standard

Date	From	To
Mar 20	2045	2087
Apr 20	2041	2083
May 20	2037	2079
Jun 20	2034	2075
Jul 20	2030	2071
Aug 20	2026	2067
Sep 20	2022	2063
Oct 20	2018	2059
Nov 20	2014	2055
Dec 20	2010	2051
Jan 21	2006	2047
Feb 21	2002	2043
Mar 21	1999	2039
Apr 21	1995	2035
May 21	1991	2031
Jun 21	1987	2027
Jul 21	1983	2023
Aug 21	1979	2019
Sep 21	1976	2016
Oct 21	1972	2012

$2066 \times 98\% =$
 $2024.68 \square 2025$

$2066 \times 102\% =$
 $2107.32 \square 2107$

Range = 2025 $\square$ 2107

Moisture
Standard

$622 \times 96\% =$
 $597.12 \square 597$

$622 \times 104\% =$
 $646.88 \square 649$

Range = 597 $\square$ 649

Standard Practices

Operation – Standardizing the Gauge:

- Check to make sure Gauge Standard counts are within limits of the Calibrated Manufacturer's Standard Counts.
- **Density = +/- 2%**
- **Moisture = +/- 4%**
- **Remember - this Range comes from the Calibration Sheets for the Gauge. The numbers from the Gauge when you Standardize must fall into this Range.**

Standard Practices

Operation – Testing:

- Verify the settings in the Gauge.
 - Proper Units – Kg/m³
 - Asphalt is always a Wet Density Reading
 - Asphalt is always tested in the Backscatter Position
 - Proper target density set in the gauge
 - Tests for DOH must be 1 Minute tests
- Find test location by using Random Numbers.
- Make sure you don't test within 30 feet of another gauge.
- Make sure Gauge is sitting on a solid area away from large metal objects such as equipment and trucks.

Gauge Operation

- Good contact between Gauge and Surface



Fill Voids

- Dry Mortar Sand
- Silica Sand

Remove
Excess

Gauge in Position



Too Much Exposure To The Gauge



Gauge Common Sense

DO NOT OPERATE NEAR :

- Large metal objects
- Equipment
- Culverts
- Power lines
- Other gauges (minimum of 30 feet)

KEEP GAUGE DRY :

- Cover control panel if possible

Specifications Section 401

Provides the requirements for determining how acceptance will be made.

1. Lot by Lot Testing
2. Roller Pass Method

Quality Control Testing

- Quality control tests are performed to allow the Contractor to control the material. These tests are not used for acceptance.
- The Contractor is responsible for quality control even when acceptance tests are not required, for example, a Roller Pass.

Acceptance Testing

- **Lot By Lot Testing - 2023 Spec Book**
 - Measured Roadway Width > 16'
 - Total New Thickness >= 1.5"
 - Total Continuous Paving Length >= ½ Mile

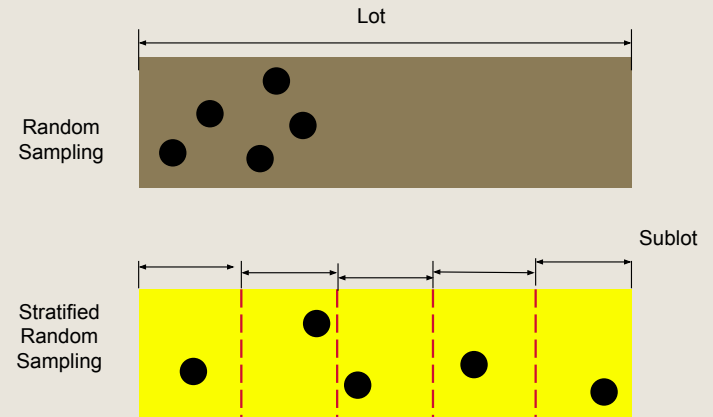
Acceptance Testing

- **Lot By Lot Projects -**
 - All Projects Bid in 2023 – 2023 Spec Book - DOH does the Acceptance Testing.
 - Projects Let in 2022 or Prior – 2022 Supplemental Specs - Contractor does the Acceptance Testing.
 - Laydown POs – 2017 Spec Book - DOH does the Acceptance Testing – No Joint Testing.

Acceptance Testing

- **Roller Pass Projects** - The Contractor is responsible for the testing while being observed by the District.
- **Roller Pass Method** –
 - Laydown POs - **MP401.05.20**
 - **2022 & Prior Projects** – 2022 Supplemental
 - **2023 Projects** – 2023 Spec Book

Lots and Sublots



Station Numbers

- Common way of tracking longitudinal distance on Projects
- The “ + ” Sign is simply a place holder – just like the comma in 10,000
- Beginning of Project (BOP) = 0+00
- End of Project (EOP) = 144+25 (Example)
- 144+25 = 14,425 Feet

Offsets

- Common way of tracking left & right distance on Projects
- On Paving Projects, typically measured from the centerline or the center joint
- Never test on the edge of the pavement – pull the gauge 1' in away from the edge
- Offsets for Joint Testing with the gauge are always 4 inches

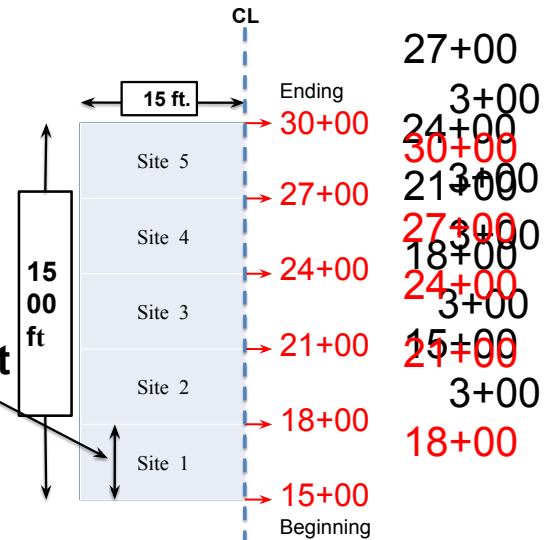
Sublot Exercise

Handbook – 5 Pages from end

Lot = 1500 ft.
5 sub lots in a lot

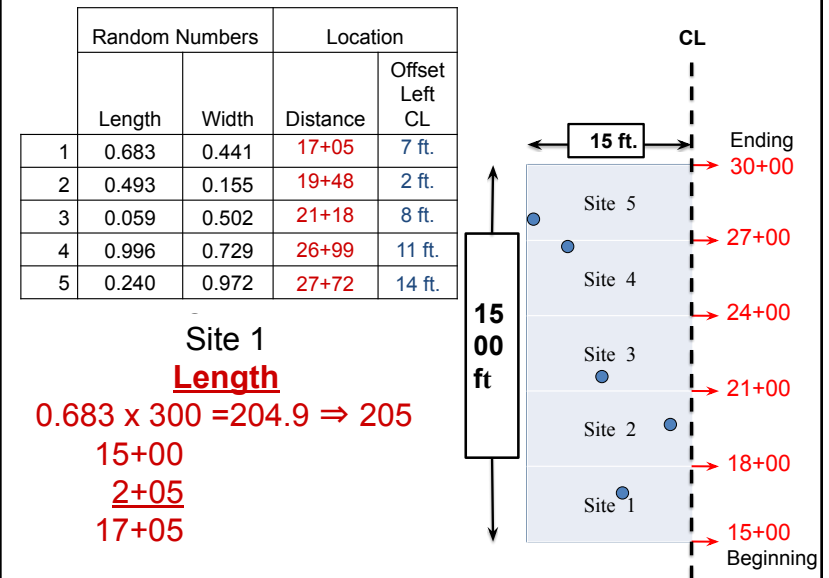
$$1500 / 5 = 300$$

Each sub lot
or test site
will represent
300 ft.



Sublot Exercise

	Random Numbers		Location	
	Length	Width	Distance	Offset Left CL
1	0.683	0.441		
2	0.493	0.155		
3	0.059	0.502		
4	0.996	0.729		
5	0.240	0.972		



Testing Process

Testing Forms

- Lot x Lot -- T401
- Gauge Comparison -- T401b
- Roller Pass -- T407
- All three are located on DOH "Toolbox" Web Page

<http://transportation.wv.gov/highways/mcst/Pages/tbox.aspx>

Gauge Comparison

Comparison of Nuclear Density Gauges In Accordance with Guidelines Contained in Section 401.6.4.1.1

2023 Gauge Verification

1. Randomly locate **3** test sites within the first **500 ft** of paving – starting **100 ft** from the take-off Joint & a minimum of **1 ft** from either edge.
2. Take 5 One-Minute readings in the same spot.
3. For each gauge used – the Range of the 5 readings cannot exceed 25 kg/m³.
4. Once all 3 locations are done average the 15 readings for each gauge to compare differences between gauges.
5. This will be used so the Contractor can determine the Density to achieve on their gauge.

T-401b Gauge Comparison Form

T-401b - Gauge Comparison
Rev 2023-04-25 401/402 West Virginia Division of Highways
Lot-By-Lot Asphalt Compaction QC-QA Comparison

Project No:	Item Number:	Density Standard:
Authorization No:	T400 Number:	QC Gauge No:
Contract ID No:	Division Gauge No:	Min. Standard:
Plant Source Code:	Min. Standard:	Density Standard:

Station/Offset:	Test	QA Tests			QC Tests		
		Site 1	Site 2	Site 3	Site 1	Site 2	Site 3
Station/Offset:	Test 1				Test 1		
Station/Offset:	Test 2				Test 2		
Station/Offset:	Test 3				Test 3		
Station/Offset:	Test 4				Test 4		
Station/Offset:	Test 5				Test 5		
The range of the 5 readings at each location shall not exceed 25 kg/m ³ . If the readings exceed this range, perform a new set of 5 readings.		Range			Range		

QA TESTS AVERAGE:	QC TESTS AVERAGE:	DIFFERENCE
		Difference = QA Gauge - QC Gauge (note +/- sign)

Technician's Name:	DATE:	TIME:
Reviewer's Name:	DATE:	TIME:

T-401b Gauge Comparison Form

Part 1 - Identification and Verification

T-401b - Gauge Comparison West Virginia Division Of Highways
Rev 2023-04-25 401/402 Lot-By-Lot Asphalt Compaction QC-QA Comparison

Project No:	STP-033-2.22	Item Number:	401002-020	Density Standard:	2479
Authorization No:	AB 2575 G	T400 Number:	2018448	QC Gauge No:	45678
Contract ID No:	1122334	Division Gauge No:	52889	Man. Standard:	2000
Plant Source Code:	XYZ.01.400	Man. Standard:	2500	Density Standard:	1991

T-401b Gauge Comparison Form

Part 2 - Repeatability & Similarity

		QA Tests			QC Tests		
		Site 1	Site 2	Site 3	Site 1	Site 2	Site 3
Station/Offset:	1+33 / 4' R	Test 1	2250		Test 1	2290	
Station/Offset:	2+59 / 8' R	Test 2	2255		Test 2	2310	
Station/Offset:	4+62 / 7' R	Test 3	2272		Test 3	2299	
Station/Offset:		Test 4	2270		Test 4	2298	
Station/Offset:		Test 5	2258		Test 5	2297	
The range of the 5 readings at each location shall not exceed 25 kg/m ³ . If the readings exceed this range, perform a new set of 5 readings.		Range:	22		Range:	20	
QA TESTS AVERAGE:		2261			DIFFERENCE		
QC TESTS AVERAGE:		2299			- 38		
					Difference = QA Gauge - QC Gauge (note +/- sign)		

Testing Process

Density Testing – Lot By Lot
2022 & Prior Projects

Lot = 1,500 Ft long by the Paving Width
Sublot = 300 Ft long by the Paving Width
Acceptance Range = 92% - 97%
Joint Density Testing on the “Hot” side

Testing Process

Density Testing – Lot By Lot
DOH Laydown POs

Lot = 1,000 Ft long by the Paving Width
Sublot = 200 Ft long by the Paving Width
Acceptance Range = 92% - 96%
Joint Density Testing is not done

Testing Process

Density Testing – Lot By Lot 2023 Projects

Lot = 1,000 Ft long by the Paving Width
Sublot = 200 Ft long by the Paving Width
Acceptance Range = 93% - 97%
Joint Density Testing on the “Hot” side

Testing Process

Density Testing – Joint Testing 2023 Projects

- Done on the “Hot” side when a second travel lane is constructed.
- 5 Random 1 Minute Wet Density Tests per Lot - 4 Inches off of the Joint.
- Done at the same Station as the Mainline Density Test

Testing Process

Density Testing – Joint Testing 2023 Projects

- Calculate the **Average Density** the same as the Mainline Densities.
- Evaluate the Lot – Pass / Fail -
Spec Range = 90% - 97%

T401 Compaction Form – April 2023 Revision

T - 401
Rev 2023-04-10

West Virginia Division of Highways
401/402 Lot-By-Lot Asphalt Compaction Form

Project Number:	Item Number:	Grade Number:
Contract ID:	Task Number:	Max Std Count:
Contract Line Item:	Target Density (A):	Density Std Count:
Plant Source Code:	Lift Thick (in):	District & County:

Date:		Acceptance Tests									
Beginning Station:	Ending Station:										
Lab Number:											
Lot Number:		A1	J1	A2	J2	A3	J3	A4	J4	A5	J5
Wet Density (B1):											
% Relative Density (C):											
Lot Evaluation: Pass/Fail											

Date:		Quality Control or Additional Acceptance Tests									
Beginning Station:	Ending Station:										
Paving Width (Feet):											
Lab Number:											
Lot Number:		A1	J1	A2	J2	A3	J3	A4	J4	A5	J5
Offset (ft or 1/2 of C):		4 inches		4 inches		4 inches		4 inches		4 inches	
Wet Density:	1										
Wet Density:	2										
Wet Density:	3										
Wet Density:	4										
Wet Density:	5										
Average Wet Density (B) = Avg (1 thru 5)											
Relative Density (N) (C)											
Lot Evaluation: Pass/Fail											
Lot % Pay (N)											

% Relative Density	90-92%	93-95%	96%	97%	98%	99%	100%	101%	102%	103%	104%	105%
Max Density % Pay	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Joint Pay Adjustment	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Technician's Name: _____

Reviewer's Name: _____

Relative Density (C) (%) = $\frac{\text{Wet Density (B)} \times 100}{\text{Target Density (A)}} \times 100\%$

Lot Pay (%) = Max Density % + Joint Adjustment

T - 401
Rev 2023-04-10

West Virginia Division Of Highways
401/402 Lot-By-Lot Asphalt Compaction Form



Project Number:		Item Number:		Gauge Number:	
Contract ID:		T400 Number:		Man Std Count:	
Contract Line Item:		Target Density (A):	2465	Density Std Count:	
Plant Source Code:		Lift Thick (in):		District & County:	

T401
Compaction
Form – April
2023 Revision

Acceptance Tests

Date:									
Begin/End Station:									
Lab Number:									
Lot Number:	A1	J1	A2	J2	A3	J3	A4	J4	A5
Wet Density (B):	2277		2288		2305		2291	2251	2295
% Relative Density (C):	92		93		94		93	91	93
Lot Evaluation: Pass/Fail			Pass		Pass		Pass	Pass	Pass

2465 - Gmm

T401
Compaction
Form – April
2023 Revision

Quality Control or Additional Acceptance Tests										
Date:										
Beginning Station:										
Ending Station:										
Paving Width (Feet):										
Lab Number:										
Lot Number:	A1	J1	A2	J2	A3	J3	A4	J4	A5	J5
Offset (Rt or Lt of CL):		4 Inches		4 Inches		4 Inches		4 Inches		4 Inches
Wet Density: 1	2266									2129
Wet Density: 2	2307									2134
Wet Density: 3	2286									2201
Wet Density: 4	2276									2176
Wet Density: 5	2288									2144
Average Wet Density: (B) = Avg (1 thru 5)	2285									2157
Relative Density (%): (C)	93									88
Lot Evaluation: Pass/Fail	Pass								Pass	Fail
Lot % Pay (%)	100									97
% Relative Density	> 97%	93-97%	92%	91%	90%	89%	88%	87%	86%	85%
Mat Density % Pay	Note 1	100%	99%	95%	91%	87%	83%	73%	63%	53%
Joint Pay Adjustment	Note 2	0%	0%	0%	0%	-1%	-3%	-9%	-15%	-21%

T401
Compaction
Form – April
2023 Revision

Roller Pass

Select 100 foot **CONTROL SECTION** – 100 Ft past the Take-off Joint.

Divide into TWO equal **SUBLOTS** (50 foot each)

Random Numbers		Location	
Length	Width	Distance	Offset (Left)
1	0.338	0.917	1+17
2	0.629	0.631	1+81

Length

0.338 x 50

16.9 ⇒ 17

Width

0.917 x 12

11.0 ⇒ 11

Measure density

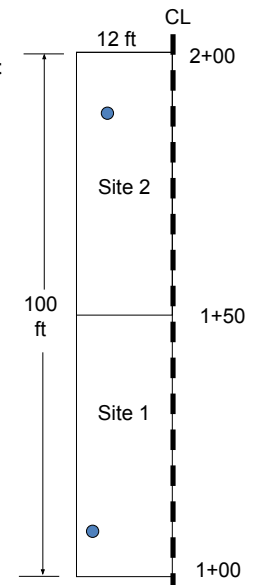
Use form T 407

1+00

1+17

Site 1

Site 2



Testing Process (2022 & 2023)

Density Testing – Roller Pass

- **Control Section** - 100 ft to 200 ft from the take-off joint. (100 Feet Long)
- 2 – 50 Ft. sublots with 1 random testing location within.
- Apply 4 Passes, take 1 Minute Test, Mark Gauge Location, record results and temperature - for each location.

Testing Process (2022 & 2023)

Control Section – Roller Pass

- Add 2 more Passes – Repeat Test in the **same spot** as the first one, record results and temperature.
- Compare second set average wet density to the first set average wet density.

Testing Process (2022 & 2023)

Control Section – Roller Pass

- Continue process – 2 Passes at a time until one of the following occur :
- Less than **5 kg/m³** increase in Average Wet Density.
- One or Both test Locations “Break Over”
- The Relative Density **exceeds 97%** of the Theoretical . Max Gravity (Gmm)
- The Mat Temperature Reaches 175 Degrees

Testing Process (2022 & 2023)

Density Testing – Roller Pass

Proving Section – Area after Roller Passes are established to validate it is the correct number.

Daily Roller Passes to be established per Project

**Roller Pass
T407
Compaction
Form -
July 2019
Revision**

T - 407
Rev 2023-04-04
West Virginia Division of Highways
401/402 Roller Pass Asphalt Compaction Form

Project Number:	Item Number:	Grade Number:
Contract ID:	ES&B Number:	Mile Station:
Contract Line Item:	Target Density:	Density Test Count:
Plant Source Code:	Lift Thick (in):	District & County:
Lift Number:	Mix Type:	Date:

Control Section	Begin Station	End Station	Site #	Station	Offset
Section Number: 1	1+00	2+00	A		
Readings after:	4 Passes	8 Passes	8 Passes	10 Passes	12 Passes
Site #	A	B	A	B	A
Mat Temperature (F)					
Wet Density (Kgm ³)					
Avg Wet Density (Kgm ³)					
Difference ^{max} (Kgm ³)					
Relative Density					

Proving Section	Begin Station	End Station
Section Number:		
Site #	1	2
Station number		
Offset		
Mat Temperature (F)		
Wet Density (Kgm ³)		
Average Wet Density ^{max}		
Avg Relative Density		
Pass/Fail		

Technician's Name:	DATE
Reviewer's Name:	DATE

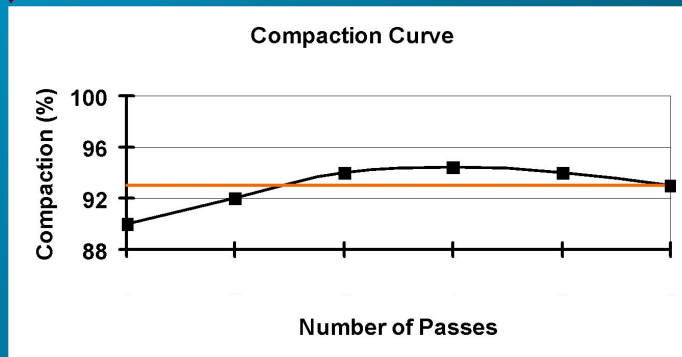
Relative Density (%) = $\frac{\text{overlaid density}}{\text{avg wet density}} \times 100\%$

Testing Process (2023)

Proving Section – Roller Pass

- **1000** Feet long beginning at the end of the Control Section.
- **5** Random Wet Density Tests
- Average Density must be 92% or greater
- Average must be within **50** Kg/m³ of the Control Section Density.

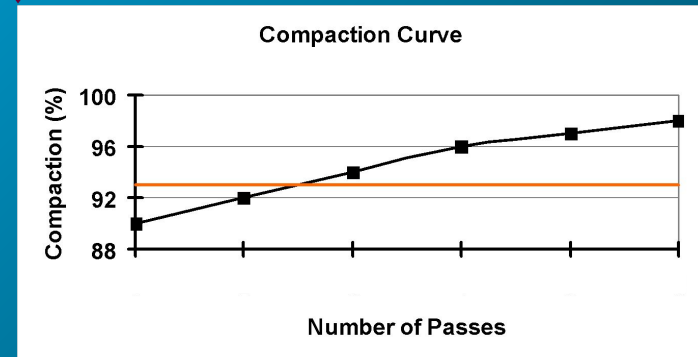
Roller Pattern Problem #1



Decreasing Temperature

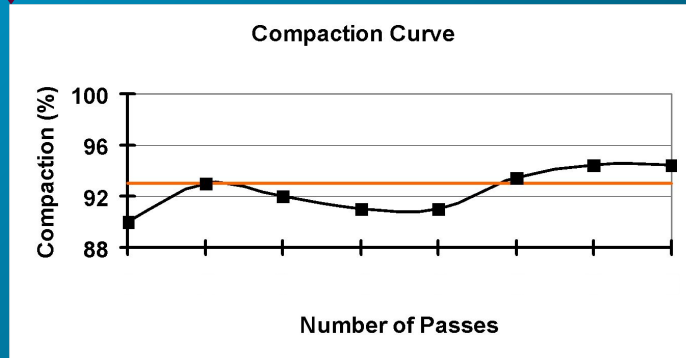
8-47

Roller Pattern Problem #2



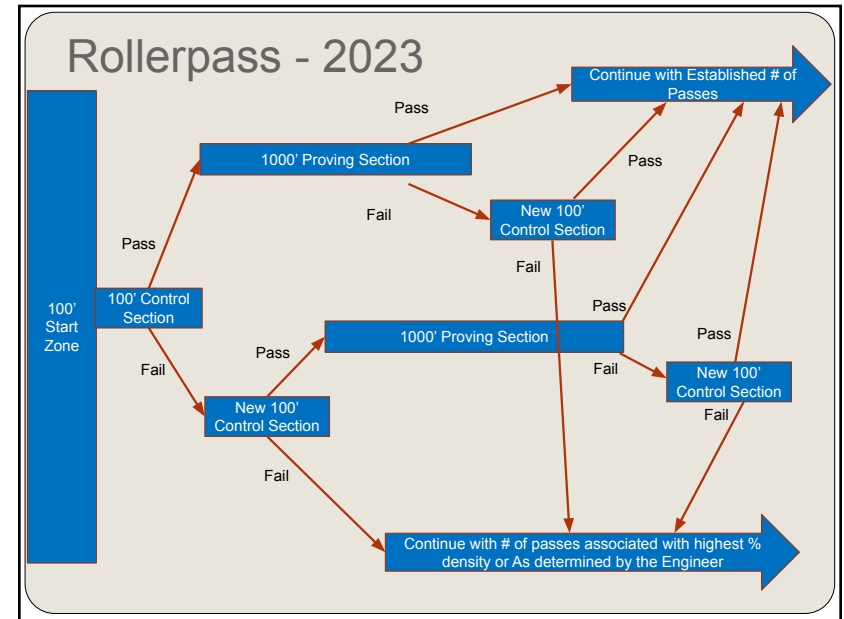
Decreasing Temperature

8-48



Decreasing Temperature

8-49



Nuclear Gauge Testing Summary

1. Use random numbers to locate test sites according to MP 712.21.26.
2. Must have a void free surface, fill voids with dry silica sand or mortar sand. Surface must be smooth and flat.
3. Take a ONE minute WET DENSITY Reading in BACKSCATTER Position.
4. Be careful that your gauge doesn't get damaged by construction equipment etc.
5. Check with District prior to Project to make sure of which testing Specs to use.

Example Tack Calculation

- Existing roadway is very old, dried, cracked, etc.
 - “Oxidized” from Table 408.11
- Prior to tack placement, dial gauge reads 450 gallons. Tack is diluted.
- Tack is placed from Sta. 0+00 to Sta. 10+00. Dial gauge reads 275 gallons after tack placement.
- Road width is an average of 12’.

Example Tack Calculation

TABLE 408.11

Condition of Existing Pavement	Application Rate (gal/sy) ^(Note 2)	
	Undiluted	Diluted (1:1) ^(Note 3)
New HMA ^(Note 4)	0.04 – 0.05	0.08 – 0.10
Oxidized HMA	0.07 – 0.10	0.13 – 0.20
Milled Surface	0.10 – 0.13	0.20 – 0.27
PC Concrete	0.07 – 0.10	0.13 – 0.20

Note 2: Application rates are for slow setting emulsions grades (SS and CSS) that contain approximately 60% asphalt material. Rapid setting emulsion grades may contain slightly higher or lower asphalt contents, but can usually be applied within the same application range.

Note 3: Dilution rate only applies to SS and CSS grades.

Note 4: Tack coat is normally not needed over a layer of new HMA that has been placed within the last few days, as long as the underlying new layer has not become dirty under traffic or from windblown dust.

408.12-METHOD OF MEASUREMENT:

No materials shall be removed from the Project for any purpose until the operation has been completed and the quantities of materials incorporated into the operations have been determined, except when authorized by the Engineer.

The quantity of "Asphalt Material" for tack coat shall be the number of gallons, prior to dilution, incorporated into the completed work. Any applicable dilution rates, shall be supplied to the Engineer by the Contractor on the material delivery ticket.

Dry sand or stone chips used as a blotter course due to excessive use of tack coat shall be considered incidental to the work.

When items for maintaining traffic are included in the Contract, they will be measured and paid as provided in Section 636.

Example Tack Calculation

■ Calculate the tack used

$$\text{PAY QTY.} = 450 \text{ gal} - 275 \text{ gal} = \underline{175 \text{ GALLONS}}$$

■ LETS CHECK THE APPLICATION RATE:

$$- (12' \times 1000') \div 9 \text{ ft}^2 \text{ per yd}^2 = 1333.33 \text{ yd}^2$$

■ Calculate Rate of Application

$$- 175 \text{ gal} \div 1333.33 \text{ yd}^2 = \mathbf{0.13 \text{ gal/yd}^2}$$

TABLE 408.11

Condition of Existing Pavement	Application Rate (gal/sy) (Note 2)	
	Undiluted	Diluted (1:1) (Note 3)
New HMA (Note 4)	0.04 – 0.05	0.08 – 0.10
Oxidized HMA	0.07 – 0.10	0.13 – 0.20
Milled Surface	0.10 – 0.13	0.20 – 0.27
PC Concrete	0.07 – 0.10	0.13 – 0.20

Contract: _____ Inspector: _____ DWR Date: 06/17/19
 Project Nbr: _____ Line Itm Nbr: 0015 Loc Seq No: 1
 Item Code & Desc: 408002-001 ASPHALT MATERIAL

West Virginia Department of Transportation
 Division of Highways
 Inspector's Bituminous Emulsion Tack Worksheet

Ver 1.1 March 2 2016

Producer / Supplier: MPC1.01.705 - Marathon Petroleum F-1 @ Catlettsburg Total Quantity Placed: 175.

Row 1 of 1 Ticket Number Original Invoice No Material Type Source of Material

OBSERVATIONS - *Comment below if any of the following are not met:*

Traffic Control and Flaggers in place ☒
 Surface temp above 40 degrees F ☒
 Surface clean prior to placement ☒
 Uniform application of tack coat ☒

Existing Pavement Condition	Target Application Rate (gal/yd ²)	
	Undiluted ☒	Diluted (1:1) ☐
New HMA ☐	0.04 - 0.05	0.08 - 0.10
Oxidized HMA ☒	0.07 - 0.10	0.13 - 0.20
Milled Surface ☐	0.10 - 0.13	0.20 - 0.27
PCC ☐	0.07 - 0.10	0.13 - 0.20

*Undiluted = 60% Residual Asphalt, Diluted = 30% Residual Asphalt

A	B	C	D	E	F	G	H	I	J
Time	Start Station	End Station	Length (ft)	Width (ft)	Area (yd ²)	Initial Reading (gal)	Final Reading (gal)	Amount Applied (gal)	Rate (gal/yd ²)
			C - B		(D x E) / 9			G - H	I / F
00:00	0 + 00	10 + 00	1000.	12.	1333.33	450.	275	175.00	.13

Remarks

Running Amount Applied 175.

EXAMPLE DWR ENTRY



Division of Highways

01/02/2024 11:00:19

Daily Work Report

Report v1

Contract: 2023080002, BIG OTTER-FRAMETOWN RD				Prime Contractor: WEST VIRGINIA PAVING, INC.	
DWR Date: 9/6/2023	Day of Week: Wednesday	Inspector: James Putnam	Agency Project Engineer: Steven Ruppert		
Seq Num: 1	DWR Status: Approved	Entered By: James Putnam	Agency Delivery Engineer: Steven Ruppert		
Last Updated Date: 9/14/2023		Last Updated By: Savannah Summers	Managing Office:		
Weather: 9		Low Temp: 66	High Temp: 91	Rainfall Amount:	SECIR Indicator:
Fed. Proj. Num: NHPP-0079(091)		State Proj. Num: S308-79-43.37 00			
Work Items: Yes	Contractors: Yes	Daily Staff: No	Attachment(s): Yes		

Remarks

Type	Remarks
00	PRIME CONTRACTOR TIME WV PAVING NO LUNCH 6:00AM TO 7:00PM = 13 HRS WVDOH PERSONNEL TIME JAMES PUTNAM 5:30AM TO 7:30PM = 13.5 HRS 1 HR TRAVEL 12.5 HRS PROJECT INSPECTION, DOCUMENTATION, ETC WVDOH PERSONNEL TIME DAKOTA RIFFLE 6:30AM TO 7:30PM = 12.5 HRS 1 HR TRAVEL 11.5 HRS PROJECT INSPECTION, ETC
02	2023 09 06 DWR 0025 SUPERPAVE ASPHALT TICKETS TY19
	2023 09 06 DWR 0025 SUPERPAVE ASPHALT DELIVERY REPORT TY19
03	LINE # 0025, ITEM 410001-030 SUPERPAVE ASPHALT BASE TY 19

Contract : 2023080002

DWR Date: 9/6/2023 , Seq. No.: 1

Page: 1 of 8



Daily Work Report

Report v1

Item Postings

Item Description	Item ID	Seq. Num	Cont Ln	Project	Category	Proj Ln	Quantity Posted	Unit	From Station - To Station Location
SUPERPAVE ASPHALT BASE CRSE, TY 19	410001-030	1	0025	2023080002	0001	0025		SY	BIG OTTER FRAMETOWN RD PROJECT

Materials:

Material Name	Installed Quantity	Unit	Source	Work Location
19 mm Asphalt Mix, Superpave		TN		

Contractor: WEST VIRGINIA PAVING, INC.

Comments: PAY FOR THIS ITEM WILL BE MADE AT A LATER DATE ON COMPLETION OF PWL SAMPLE TESTING.

AT THE FOLLOWING LOCATIONS, THE CONTRACTOR USES A TRANSFER MACHINE AND A RUBBER TIRE PAVER TO PLACE ASPHALT BASE FOLLOWED BY 3 STEEL DRUM ROLLERS FOR COMPACTION. TERRADON TECHNICIAN, JUSTIN WHITTINGTON, ON SITE FOR DENSITY. WV PAVING, JAMEY PROPPS ON SITE FOR PWL CORE DRILLING. WVDOT TECHNICIAN DAVID SCHOOLCRAFT ON SITE.

(PAYMENT WILL BE REQUESTED FOR A 12FT LANE)

SB SLOW LANE

2611+75 TO 2596+70 = 1505

EQUALITY

2595+78 TO 2516+35 = 7943

1505 + 7943 = 9448

 $((9,448 \times 12 = 113,376) / 9 = 12,597.333)$

NB SLOW LANE

2516+35 TO 2522+80 = 645

 $((645 \times 12 = 7,740) / 9 = 860)$

12,597.333 + 860 = 13,457.333 SY



Daily Work Report

Report v1

PAY QUANTITY = 0 SY

TRAFFIC CONTROL DEVICE	636011-001	1	0140	2023080002 0001	0140	0 UN	BIG OTTER FRAMETOWN RD PROJECT
------------------------	------------	---	------	-----------------	------	------	--------------------------------------

Materials:

Material Name	Installed Quantity	Unit	Source	Work Location
Traffic Control Devices	0	EA		

Contractor: HIGHWAY SAFETY, INC.

Comments: ALL TRAFFIC CONTROL DEVICES ARE PROPERLY LOCATED THROUGHOUT THE PROJECT. THE SIGNS ARE CLEAN, LEGIBLE, AND CLEARLY VISIBLE. SPEED TRAILERS AND ARROW BOARDS ARE ALSO UTILIZED IN EACH LANE AS PART OF THE REQUIRED SPECIFICATIONS FOR THE TRAFFIC CONTROL SET UP AS LISTED PER THE PLAN SHEET. REFER TO THE DAILY CHECK LIST FOR ADDITIONAL INFORMATION.

PAY QUANTITY = 0 UN

TRAFFIC DIRECTOR	636014-002	1	0155	2023080002 0001	0155	11.5 HR	BIG OTTER FRAMETOWN RD PROJECT
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Materials:

Material Name	Installed Quantity	Unit	Source	Work Location
---------------	--------------------	------	--------	---------------

Contractor: WEST VIRGINIA PAVING, INC.

Comments: CONTRACTOR USES A TRAFFIC DIRECTOR ON THIS DATE FOR THE SAFETY OF THE TRAVELING PUBLIC. PATROL VEHICLE FOLLOWED THE OPERATIONS THROUGHOUT THE PROJECT AS PER THE CONTRACTOR'S REQUEST. THE OFFICER ON DUTY THIS DATE IS BRIAN YOUNG.

HOURS ON SITE 7:00AM TO 6:30PM = 11.5 HRS

PAY QUANTITY = 11.5 HR

ELECTRIC ARROW	636021-001	1	0160	2023080002 0001	0160	2 DA	BIG OTTER FRAMETOWN RD PROJECT
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Materials:

Material Name	Installed Quantity	Unit	Source	Work Location
---------------	--------------------	------	--------	---------------

Contract : 2023080002

DWR Date: 9/6/2023 , Seq. No.: 1

Page: 5 of 8



Division of Highways

01/02/2024 11:00:19

Daily Work Report

Report v1

Comments: CONTRACTOR UTILIZES 32 TYPE B WARNING LIGHTS. ALL TYPE B WARNING LIGHTS ARE PLACED PROPERLY. ALL TYPE B LIGHTS ARE ON AND WORKING PROPERLY. AS PER THE PLANS, B LIGHT PAYMENT WILL BE REQUESTED FOR PLACEMENT ON ROAD WORK 2 MILES SIGNS, ROAD WORK 1 MILE SIGNS, AND 55 MPH SIGNS.

ALL B-LIGHTS ARE WORKING THIS DATE

PAY QUANTITY = 32 DA

SHADOW VEHICLE	636028-002	1	0175	2023080002	0001	0175	MO	BIG OTTER FRAMETOWN RD PROJECT
----------------	------------	---	------	------------	------	------	----	--------------------------------------

Materials:

Material Name

Installed Quantity

Unit

Source

Work Location

Contractor: WEST VIRGINIA PAVING, INC.

Comments: CONTRACTOR UTILIZES A SHADOW VEHICLE WHILE PERFORMING WORK. I PREVIOUSLY ADVISED WV PAVING TO HAVE THE SHADOW VEHICLE SET UP TO MEET SPECIFICATIONS LISTED IN THE STANDARD DETAILS. THE SHADOW VEHICLE HAS BEEN EQUIPPED WITH FLASHING BEACONS AND A TRUCK MOUNTED ATTENUATOR. SPOKE WITH VERN WALKER, WV PAVING, ABOUT HAVING 2 SHADOW VEHICLES ON THE PROJECT WHEN PROJECT SCHEDULED FOR 1 AS PER THE PLANS. WV PAVING AGREED HAVING PAYMENT FOR THE USE OF 1 UNLESS 2 BECOMES NEEDED. ONLY 1 IS BEING UTILIZED AT THIS TIME.

PUT IN SERVICE ON 07/25/2023. NEXT PAYMENT REQUEST 9/25/2023

PAY QUANTITY = 0 MO

SPEED MONITORING TRAILER	636030-001	1	0180	2023080002	0001	0180	2 DA	BIG OTTER FRAMETOWN RD PROJECT
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Materials:

Material Name

Installed Quantity

Unit

Source

Work Location

Traffic Control Devices

0

EA

Contractor: HIGHWAY SAFETY, INC.

Comments: THERE ARE 2 SPEED MONITORING TRAILERS PLACE ON THIS PROJECT. ONE PLACED ON THE SB SIDE OF THE PROJECT AND ONE ON THE NB SIDE OF

Contract : 2023080002

DWR Date: 9/6/2023 , Seq. No.: 1

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WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
INSPECTOR'S ASPHALT PAVEMENT WORKSHEET

AUTH NO: _____	PROJECT NO: _____	IDR NO: _____
LINE NO: _____	ITEM NO: _____	DATE: _____

LOAD	TICKET #	TONS / MG	CUM. TOTAL	LOAD	TICKET #	TONS / MG	CUM. TOTAL
1				25			
2				26			
3				27			
4				28			
5				29			
6				30			
7				31			
8				32			
9				33			
10				34			
11				35			
12				36			
13				37			
14				38			
15				39			
16				40			
17				41			
18				42			
19				43			
20				44			
21				45			
22				46			
23				47			
24				48			

REMARKS: _____

TOTAL Tons (Mg) THIS DATE: _____ INSPECTOR: _____

5/19/2009

Signature

ROLLER PASS DATA _____ PCF SPEED OF ROLLER _____ MPH TYPE OF ROLLER _____
 ROLLER PASS DATA _____ PCF SPEED OF ROLLER _____ MPH TYPE OF ROLLER _____
 PLAN THICKNESS _____

CALCULATION OF APPLICATION RATE (ONE CALCULATION PER EACH 762 m (2500 LF))

BEGIN STATION						
END STATION						
WIDTH						
Sm (SY)						
Mg (TONS)						
Mg/Sm (LB/SY)						

OBSERVED SIMILARITY TESTS (ONE OBSERVATION PER EACH 305 m (1000 LF))

LOT NUMBER						
TEST NUMBER						
Mg/Cm (LB/CY)						
STATION						

MAT THICKNESS (PRIOR TO COMPACTION) & MAT TEMPERATURE (AT TIME OF FINAL COMPACTION PASS) CHECKS (ONE CHECK PER 305 m (1000 LF))

TIME						
MAT TEMPERATURE						
MAT THICKNESS						
STATION						

TIME						
MAT TEMPERATURE						
MAT THICKNESS						
STATION						

ROLLER SPEED CHECKS (FOUR CHECKS - TWO AM AND TWO PM)

TIME						
ROLLER SPEED						

- CHECKS ☐ VERIFIED PRIOR TO PLACEMENT CHECKS ☐ VERIFIED DURING PLACEMENT
- | | |
|---|---|
| ☐ SEQUENCE OF OPERATION VERIFIED | ☐ TRUCKS COVERED AND INSULATED WITH NO OIL LEAKS OR DAMAGED GSEDS |
| ☐ TRAFFIC CONTROL DEVICES AND FLASHERS IN PLACE | ☐ OPERATION CONTINUOUS AND PAVING SPEED COMPATIBLE TO PLANT PRODUCTION |
| ☐ WHEEL JOINTS OUT AND POTHOLES CORRECTED | ☐ VIBRATING SCREEN ON AND TEXTURE OF MAT CORRECT |
| ☐ SURFACE CLEAN AND STRINGLINE PLACED | ☐ CORRECT ROLLING SEQUENCE BEING USED |
| ☐ ROLLER(S) AND PAYER(S) VERIFIED FOR COMPLIANCE | ☐ STRAIGHT EDGE CHECKS BEING MADE |
| ☐ CONTRACTOR'S DENSITY TECHNICIAN ON SITE | ☐ COMPLIANCE WITH QUALITY CONTROL PLAN MAINTAINED |
| ☐ LONGITUDINAL JOINTS FINISHED / NO OVERLAPPED | ☐ TEMPERATURE OF MATERIAL RECORDED ON TICKETS ONCE PER HOUR MINIMUM |

AIR TEMP _____ °C (°F) AT (TIME) _____
 AIR TEMP _____ °C (°F) AT (TIME) _____

BASE TEMP _____ °C (°F) AT TIME _____
 BASE TEMP _____ °C (°F) AT TIME _____



Project Number:		Item Number:		Gauge Number :	
Contract ID:		T400 Number:		Man Std Count :	
Contract Line Item:		Target Density (A):		Density Std Count :	
Plant Source Code:		Lift Thick (in) :		District & County:	

Acceptance Tests										
Date:										
Begin/End Station:										
Lab Number:										
Lot Number:	A1	J1	A2	J2	A3	J3	A4	J4	A5	J5
Wet Density (B) :										
% Relative Density (C):										
Lot Evaluation: Pass/Fail										

Quality Control or Additional Acceptance Tests										
Date:										
Beginning Station:										
Ending Station:										
Paving Width (Feet):										
Lab Number:										
Lot Number:	A1	J1	A2	J2	A3	J3	A4	J4	A5	J5
Offset (Rt or Lt of CL):		4 Inches		4 Inches		4 Inches		4 Inches		4 Inches
Wet Density:	1									
Wet Density:	2									
Wet Density:	3									
Wet Density:	4									
Wet Density:	5									
Average Wet Density: (B) = Avg (1 thru 5)										
Relative Density (%): (C)										
Lot Evaluation: Pass/Fail										
Lot % Pay (%)										

% Relative Density	> 97%	93-97%	92%	91%	90%	89%	88%	87%	86%	85%
Mat Density % Pay	Note 1	100%	99%	95%	91%	87%	83%	73%	63%	53%
Joint Pay Adjustment	Note 2	0%	0%	0%	0%	-1%	-3%	-9%	-15%	-21%

Technician's Name:	PRINT	REMARKS
	SIGN	
	Cert #.: DATE	
Reviewer's Name:	PRINT	
	SIGN	
	Cert #: DATE	

Note 1: See Notes in Table 401.13.3A
Note 2: See Notes in Table 401.13.3B
Note3: DO NOT round numbers until final answer.
Note 4: Report all Densities in kg/m3

$$\text{Relative Density(C)(\%)} = \frac{\text{Average Density (B)}}{\text{Target Density (A)}} * 100\%$$

$$\text{Lot Pay(\%)} = \text{Mat Density \%} + \text{Joint Adjustment}$$



Project Number:		Item Number:		Gauge Number :	
Contract ID:		T400 Number:		Mfr Std Count :	
Contract Line Item:		Target Density:		Density Std Count :	
Plant Source Code:		Lift Thick (in):		District & County:	
Lab Number:		Mix type:		Date:	

Control Section		Begin Station	End Station		Site #	Station	Offset
Section Number:	1	1+00	2+00		A		
					B		
Readings after:	4 Passes	6 Passes	8 Passes	10 Passes	12 Passes		
Site #	A	B	A	B	A	B	A
Mat Temperature (F)							
Wet Density (Kg/m³)							
Avg Wet Density (Kg/m³)							
Difference ^{Note 1} (Kg/m³)							
Relative Density							

Proving Section		Begin Station	End Station	
Section Number:				
Site #	1	2	3	4
Station number				
Offset				
Mat Temperature (°F)				
Wet Density (Kg/m³)				
Average Wet Density ^{Note 2}				
Avg Relative Density			Pass/Fail	

Technician's Name:	PRINT	REMARKS
	SIGN	
	Cert #.: DATE	
Reviewer's Name:	PRINT	
	SIGN	
	Cert #: DATE	

Note 1: (Less than 5kg/m³ increase)
Note 2: Minimum 92%, Shall be within ± 50 kg/m³ of Control
Note 3: DO NOT round numbers until final answer.

$$Relative\ Density(\%) = \frac{Overlay\ Density}{Avg\ Wet\ Density} * 100\%$$

Problem	Causes																																										
	Cold Mix Temperature	Variation of Temperature of Mix	Moisture in Mix	Mix Segregation	Improper Mix Design (Asphalt)	Improper Mix Design (Aggregate)	Parking of Roller on Hot Mat	Reversing or Too Rapidly Turning Rollers	Improper Roller Operation	Improper Base Preparation	Truck Holding Brakes	Trucks Bumping Finisher	Improper Mat Thickness for Aggregate Size	Improper Joint Overlap	Sitting Long Period Between Loads	Grade Reference Inadequate	Grad Control Wand Bouncing on Reference	Grade Control Hunting (Sensitivity too High)	Grade Control Mounted Incorrectly	Vibrators Running Too Slow	Screed Extensions Installed Incorrectly	Screed Starting Blocks Too Short	Incorrect Nulling of Screed	Kicker Screws Worn Out or Mounted Incorrectly	Feed Gates Set Incorrectly	Running Hopper Empty Between Loads	Moldboard on Strikeoff Too Low	Cold Screed	Screed Plates not Tight	Screed Plates Worn Out or Warped	Screed Riding on Lift Cylinders	Excessive Play in Screed Mechanical Connection	Over correction of Thickness Control Screws	Too Little Lead Crown in Screed	Too Much Lead Crown in Screed	Finisher Speed Too Fast	Feeder Screws Overload	Fluctuating head of material					
Wavy Surface – Short Waves (Ripples)		×		×	×	×			×	×	×						✓	✓	✓							✓			✓			✓	✓				✓	✓					
Wavy Surface – Long Waves		×		×			×				×	×			✓				✓							✓						✓	✓					✓	✓				
Tearing of Mat – Full Width		×	×		×	×							×																✓														
Tearing of Mat – Center Streaks																								✓					✓					✓									
Tearing of Mat – Outside Streaks																					✓				✓		✓		✓						✓								
Mat Texture Nonuniform		×	×		×	×				×			×		✓						✓			✓			✓		✓								✓	✓					
Screed Marks											×	×																		✓													
Screed Not Responding to correction													×						✓					✓						✓			✓	✓									
Auger Shadows						×																															✓						
Poor Precompaction										×			×							✓																✓							
Poor Longitudinal Joint	✓								×					✓			✓	✓	✓										✓										✓				
Poor Transverse Joint									×													✓	✓							✓								✓					
Transverse Cracking (Checking)			×	×		×			×	×																																	
Mat Shoving Under Roller			×	×		×		×	×	×																																	
Bleeding or Fat Spots in Mat									×																																		
Roller Marks								×	×	×																																	
Poor Mix Compaction							×	×	×	×																																	

✓ Indicates a paver problem; × indicates a problem to be investigated

There can be multiple causes of problems, investigate each one.